Hydrologic Assessment of the Reservoir Brook Watershed & Functional Assessment of Monkey Pond, Meredith, NH

A Final Report Prepared for the Lake Winnipesaukee Association & the Town of Meredith



Dubois & King, Inc.

January 14, 2021

With funding from a NHDES Local Source Water Protection Program Grant under the Drinking Water State Revolving Fund through a grant from the U.S. Environmental Protection Agency (EPA), and with supplemental funding from the Town of Meredith



LAKE WINNIPESAUKEE ASSOCIATION Keep Winni Blue



lenality

NEW HAMPSHIRE



Introduction and Overview

The Reservoir Brook subwatershed is part of the Lake Waukewan watershed, for which a watershed management plan was completed in 2016. In order to determine drainage flow and assess pollutant load, the 13 sq. mi. Waukewan watershed was delineated into 21 subwatersheds, 5 for Lake Winona, and 16 for Lake Waukewan. Of the 16 subwatersheds in the Lake Waukewan watershed, the Reservoir Brook subwatershed, located at the southern end of Lake Waukewan, was identified as the largest of the catchments directly contributing nutrient loading to the lake. Predictive modeling from the 2016 watershed plan shows this subwatershed contributes the second highest amount of phosphorus export to Lake Waukewan [second only to inputs from the Snake River, which includes the entire upstream Lake Winona watershed]. Reservoir Brook, the primary tributary in the subwatershed, drains to Monkey Pond, a 1-acre wetland located on a town-owned parcel immediately adjacent to Lake Waukewan.

The hydrologic assessment of the Reservoir Brook watershed and evaluation of the functionality of Monkey Pond is being funded by a NHDES Local Source Water Protection Program Grant under the Drinking Water State Revolving Fund through a grant from the U.S. Environmental Protection Agency (EPA) and supplemental funding provided by the Town of Meredith.

The purpose of this project is to evaluate the hydrologic characteristics of the catchment and conduct a functional assessment of the Monkey Pond wetland. Through this study, the Lake Winnipesaukee Association and the Town of Meredith seek to answer the following questions; 1) Where does the water come from? 2) What are the hydrological characteristics of the water entering Monkey Pond? and 3) How is Monkey Pond functioning at present? Is dredging appropriate? Is it a good candidate for wetland enhancement?

Monkey Pond includes three tributary inflows including that of Reservoir Brook. Additionally, there are several storm drains and one municipal parking lot that outlet directly into the pond. The pond is hydrologically connected to Lake Waukewan via two culverts beneath a railroad causeway that separates it from the lake. The rail line has a history of flood-related failures in the immediate area. The pond has silted in over time and has been dredged at least twice in the past.

The following tasks were completed as part of this evaluation and summarized in this report:

- 1. Identify and map existing conditions in the Reservoir Brook subwatershed;
- 2. Conduct a drainage analysis and hydrologic assessment of the Reservoir Brook watershed;
- 3. Conduct a wetland evaluation and functional assessment of Monkey Pond; and
- 4. Present findings to the Waukewan Watershed Advisory Committee and Town of Meredith.

Identify and Map Existing Conditions in the Reservoir Brook Watershed

The Reservoir Brook watershed was delineated into 10 catchments to reflect the drainage patterns observed and to inform the nutrient loading and hydraulic models. Mapping was separated into 3 maps for clearer identification for use in analyses. Showing soils, utilities, parcels, streams, etc. on one map proved to be too cluttered. Map 1 identifies the subcatchment areas that were delineated to different points of interest or culverts in the subwatershed. Map 2 provides additional detail depicting National Wetlands Inventory mapped areas, conservation parcels, and contours. Map 3 identifies parcel lots that are developed and parcels that are vacant. The developed parcels are categorized between those on septic systems and those on town sewer. The various maps are attached in Appendix A.

Modeling results indicate that the majority of the phosphorus contribution to Monkey Pond is from the main drainage stem of Reservoir Brook beginning with Basin 1, which contains Reservoir Pond and then flows into Basin 2, then into Basin 5, and then into Basin 9. The cumulative total phosphorus (TP) load from Basin 9 is approximately 18.5 kg per year. This represents approximately half (48%) of the total calculated load to Monkey Pond. The second highest load, approximately 9.9 kg TP, comes from Basin 8, which includes the loading from Basins 3 and 4. Of all the basins, Basin 5 contributes the highest individual TP load, approximately 15.6 kg, due to it having the most developed land of all the basins at 17.4 hectares.

A detailed summary of this task can be found in the memorandum to Bess Morrison at NHDES from Pat Tarpey at Lake Winnipesaukee Association dated 12/21/2020 and included as Appendix B.

Conduct a Drainage Analysis and Hydrologic Assessment of the Reservoir Brook Watershed

A watershed level drainage analysis was completed using HydroCAD computer modeling software. The software utilizes Natural Resources Conservation Service (NRCS) Technical Release 55 (TR-55) unit hydrograph methodology to predict stormwater flows for different rainfall events and various locations in the watershed. The evaluation reviewed seven (7) drainage culverts in the watershed that were selected based on significance to the Town and routing of the watershed. Results of the culvert evaluation highlighted significant deficiencies with hydraulic capacity for most storm events. The table below summarizes the culvert evaluation. A detailed summary of the Drainage Analysis and Hydrologic Assessment work can be found in Appendix C.

Table: Culvert Evaluation Summary

Culvert Location	Estimated Discharge Capacity	Pass 10 yr Storm	Pass 25 yr Storm	Pass 50 yr Storm	Pass 100 yr Storm	Notes
Reservoir Brook at Reservoir Road	205.67 cfs	Yes	No	No	No	48" Concrete Pipe, s=0.0147 ft/ft
Reservoir Brook at NH Route 104	1271.3 cfs	Yes	Yes	Yes	Yes	72" Concrete Pipe, s=0.0645 ft/ft
Reservoir Brook at Waukewan St. (Eastern Culvert into Monkey Pond)	30.24 cfs	No	No	No	No	48" Corrugated Metal, s=0.0016 ft/ft, pipe is sediment filled to 50%.
Waukewan St - Central Culvert into Monkey Pond	9.59 cfs	No	No	No	No	18" Plastic, s=0.0083 ft/ft
Waukewan St - Western Culvert into Monkey Pond	28.33 cfs	No	No	No	No	18" Plastic, s=0.0727 ft/ft
Eastern Railroad Culvert - Monkey Pond into Lake Waukewan	67.23 cfs	No	No	No	No	48" Steel Pipe, s=0.020 ft/ft, Lake Waukewan tailwater during normal lake levels
Western Railroad Culvert - Monkey Pond into Lake Waukewan	85.92 cfs	No	No	No	No	48" Concrete Pipe, s=0.0147 ft/ft, Lake Waukewan tailwater during normal lake levels

Conduct a Wetland Evaluation and Functional Assessment of Monkey Pond

DuBois & King, Inc. used the New Hampshire Method for Inventorying and Evaluating Wetlands in New Hampshire (2015) to assess the functions of values of the wetland within Monkey Pond. Twelve functions are evaluated and scored on a quantitative scale to assist with ranking of wetlands and for use as a baseline against which to measure changes resulting from future development or restoration in the watershed above.

A detailed summary of the Wetland Evaluation can be found in Appendix D.

Findings and Recommendations

The evaluation of Monkey Pond highlighted several functions that Monkey Pond is currently providing with regards to Water Quality improvement in the watershed. Specifically, noted were the benefits associated with sediment trapping, nutrient transformation, and shoreline anchoring. A good diversity of native wetland vegetation helps to provide these functions, as faster flowing water enters the wetland from Reservoir Brook and is slowed down and fanned out as it comes into Monkey Pond and the vegetated areas of the wetlands. Considering the watershed draining to Monkey Pond, there is limited volume available in the wetland to address flood storage. The pond has two outlet pipes under the railroad tracks discharging to Lake Waukewan. These pipes are limited in their capacity to pass larger storm events. Full-scale dredging of the pond to increase storage capacity for sediment is not recommended, as it will result in a loss of vegetation in the wetland that is providing valuable functions. Loss of vegetation will reduce the capability of the pond to trap sediment by fanning out flows and slowing velocities which allow for settling. There would be less capacity to sequester nutrients through their uptake by said vegetation. Wetland enhancement projects recommended for Monkey Pond should focus on how water is managed as it enters and exits the wetland. Entrance locations or pipe culverts should be designed to provide adequate hydraulic capacity for their respective watershed catchment areas. Plunge pools or sediment traps at these locations should be considered to encourage sediment trapping. Additional consideration should be given to areas that allow for routine maintenance and removal of accumulated sediments. Also, BMPs that remove sediment prior to reaching Monkey Pond should be considered. Any work within the jurisdictional wetland areas will require permitting through NH Department of Environmental Services and the Army Corps of Engineers. New permits for dredging Monkey Pond are not likely to be issued since it would be seen as a temporary solution and does not address the source of sediment coming into the pond.

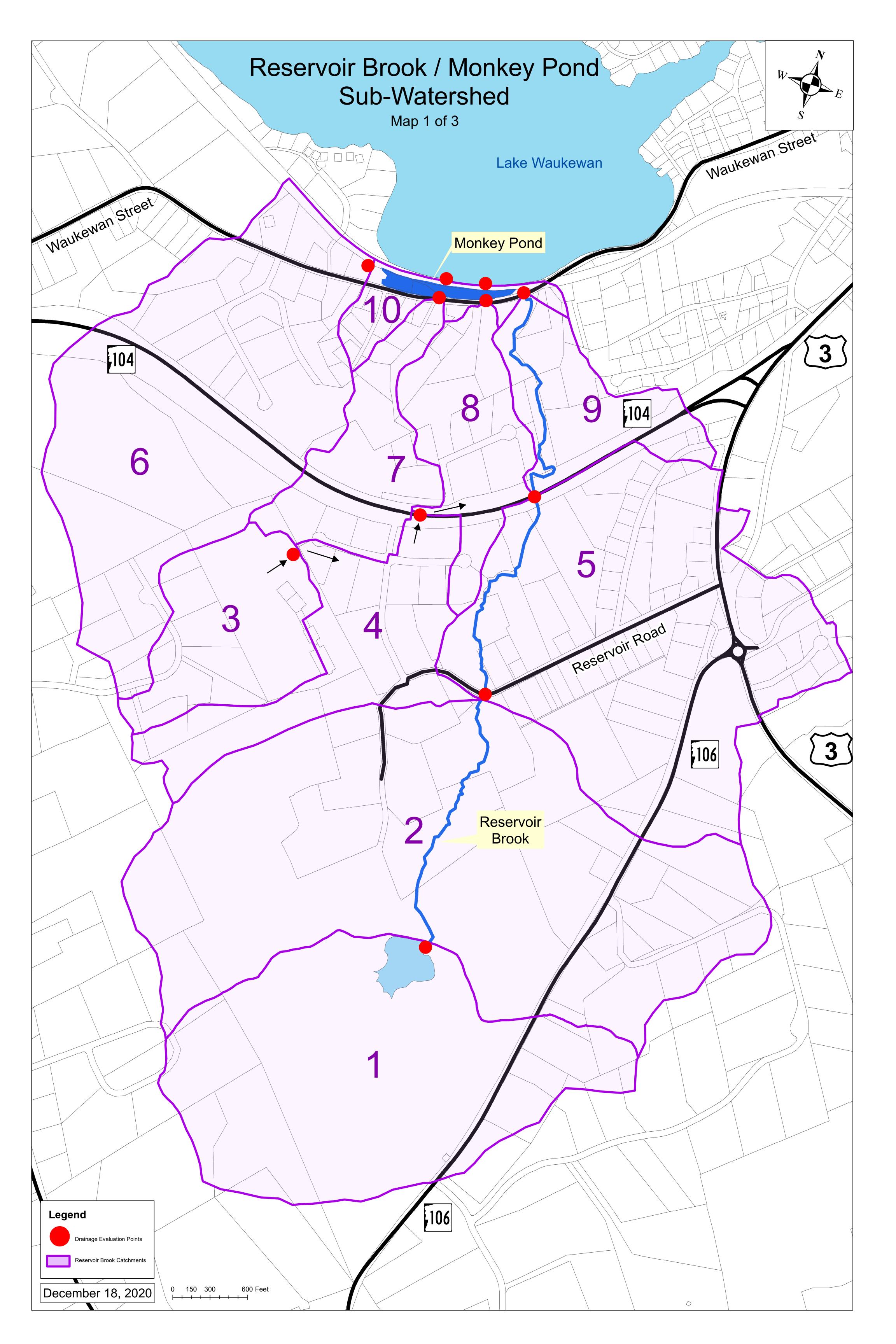
Recommendations to address sediment sources in the Watershed must also be considered. These recommendations include:

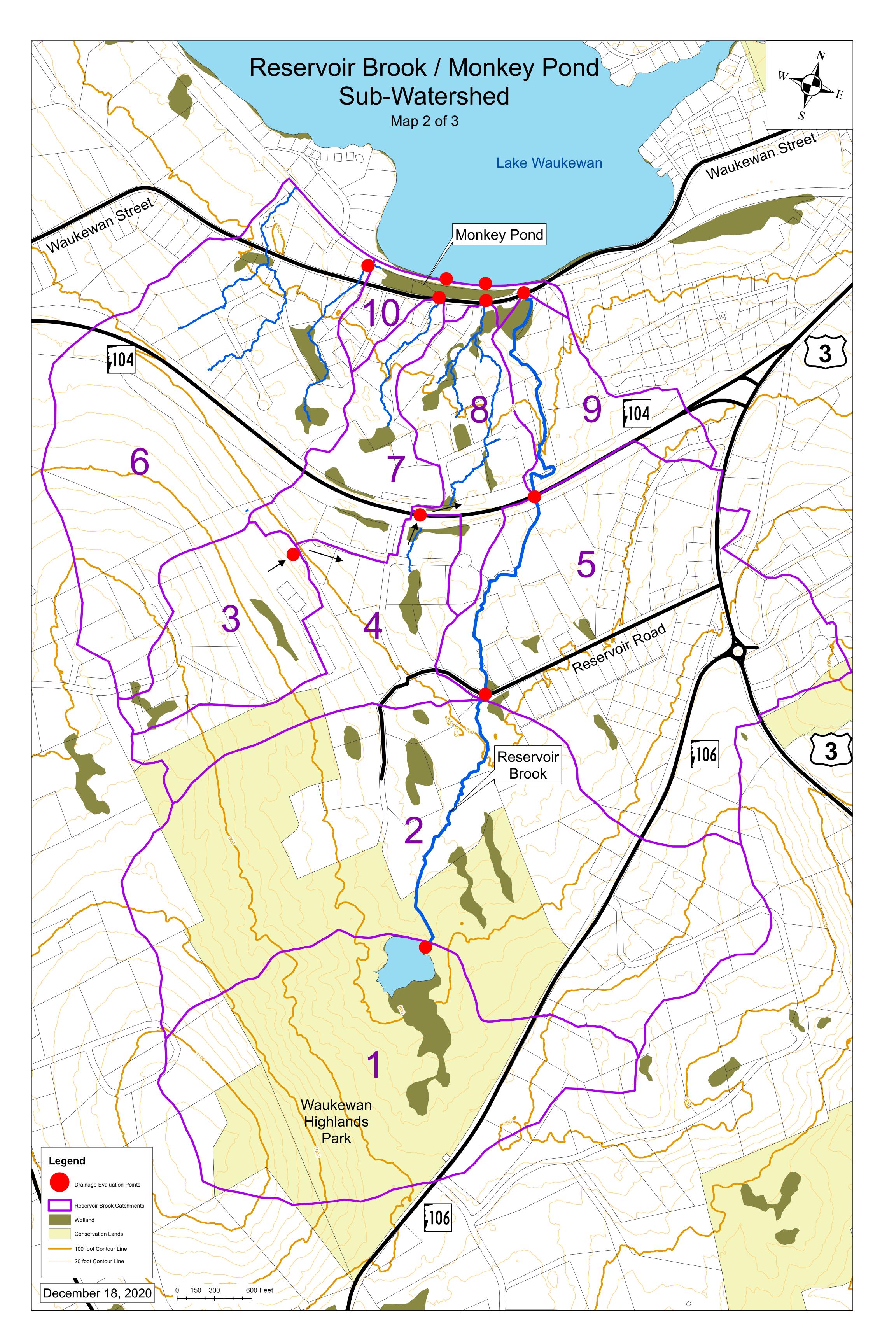
The culverts reviewed in this study were found to be hydraulically under-sized for larger storm events. Evidence of erosion around these culverts was observed and can be attributed to higher velocities where drainage areas are constrained by undersized culverts. Undersized culverts along Reservoir Brook at Waukewan Street and at Reservoir Road should be properly designed for high flows and low flows to minimize erosion in those areas. Any proposed replacement should include a more detailed hydraulic review for the culvert and a design that meets current NHDES permitting requirements. The Town should continue to communicate with the NH Department of Transportation about the culvert under Route 104 at Reservoir Brook to see that damaged headwalls are repaired and monitor erosion at that crossing. New developments and existing sites subject to review by the Planning Board should include stormwater management plans and stormwater treatment practices to address runoff from private development. Stormwater treatment practices and maintenance of these practices need to be identified during the approval process with enforcement mechanisms to ensure compliance with maintenance commitments. We encourage the Planning Board to utilize third-party review of proposed stormwater infrastructure during the development process for compliance with Town regulations and industry standards.

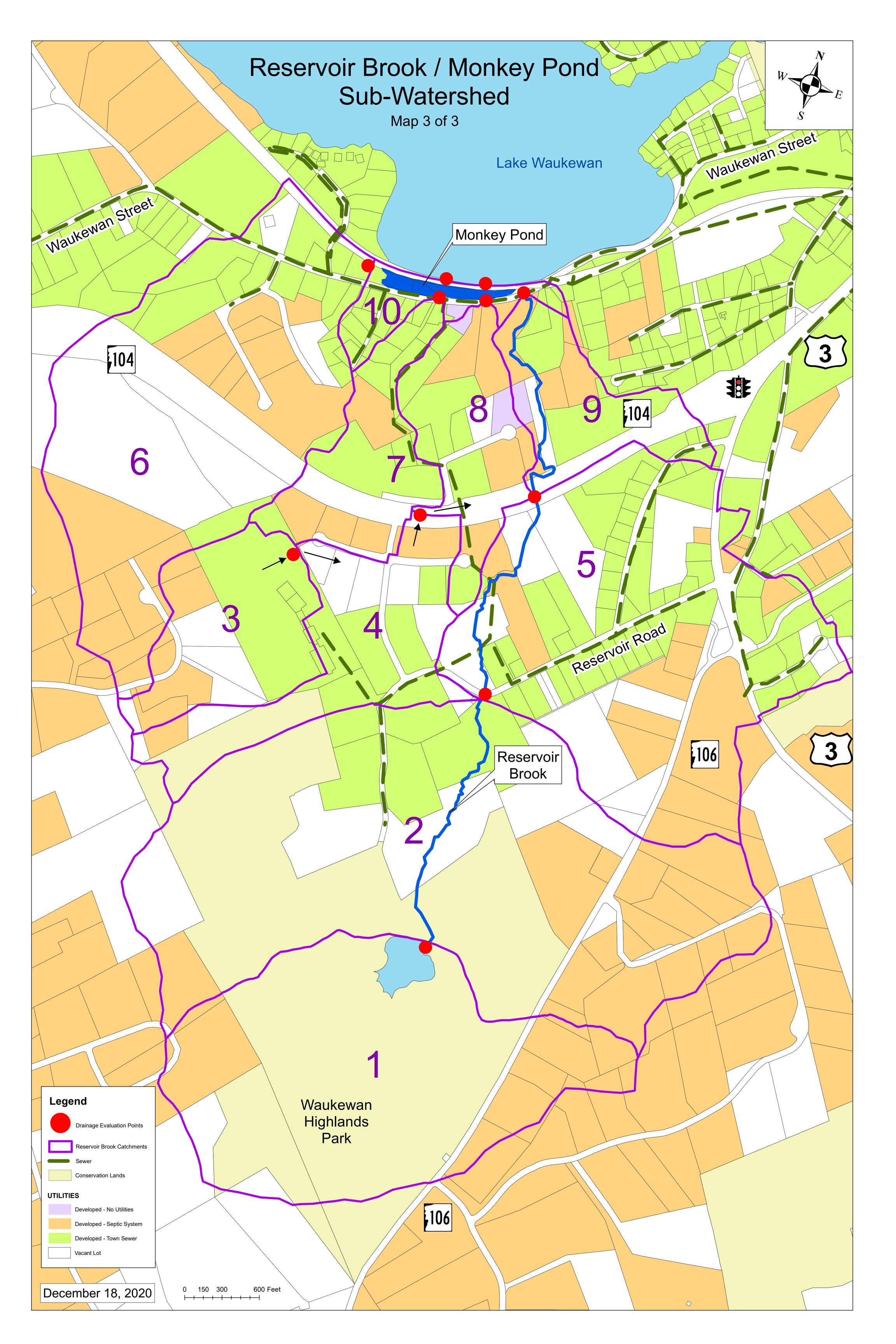
Existing developments and associated stormwater structural practices that currently exist in the watershed should be evaluated for efficacy and whether these practices are functioning according to their design intent. During our evaluation, several stormwater features were noted but could not be fully evaluated given the overall watershed project scope and budget. When opportunities present themselves, such as redevelopment or actions that would require planning board approval for these parcels, the Town should look to review these existing stormwater practices within the watershed.

The Town should review maintenance of roadside swales and ditches within the watershed and make improvements to reduce erosion in these areas. Improvements for roadside swales may include expanding ditches to accommodate larger storm events and reducing velocities, or installing surface treatments (rip rap armoring) designed to withstand higher velocities. Check dams and sediment removal maintenance should also be considered along roadside ditches. The Town should monitor sumps associated with the catch basins and closed drainage system along Waukewan Street adjacent to Monkey Pond. A review of sediment accumulation and catch basin cleaning frequency is suggested.

Appendix A







Appendix B

MEMORANDUM

To: Bess Morrison, NHDES

From: Pat Tarpey, Lake Winnipesaukee Association

Subject: Summary Report on the mapping of existing conditions and nutrient modeling for Reservoir Brook

Date: 12/21/2020

This memo provides the detailed maps of existing conditions in the Reservoir Brook watershed as well as the final results from the nutrient model used to estimate the nutrient load for each catchment in the Reservoir Brook watershed, a subwatershed of Lake Waukewan. The estimation of nutrient load is part of Task 1 of the project: *Hydrologic Assessment of the Reservoir Brook Watershed and Evaluation of the functionality of Monkey Pond.*

Task 1: Identify and map existing conditions in the Reservoir Brook watershed.

Deliverables: Detailed maps of the Reservoir Brook watershed and a summary report of modeled nutrient loads.

Watershed Delineation

The Reservoir Brook watershed was delineated into 10 catchments to reflect the drainage patterns observed and to inform the nutrient loading and hydraulic models. Mapping was separated into 3 maps for clearer identification for use in analyses. Showing soils, utilities, parcels, streams, etc. on one map proved to be too cluttered. The various maps are shown in Figures 1 through 4.

Water Routing

The water routing determined in the catchment delineations and drainage analysis is shown in Table 2. The Basin in the left-hand column passes through basin in column below if indicated by a 1. For example, drainage from Basin 1 flows to Basin 2, which then flows to Basin 5 to Basin 9 and finally into Basin 10 (Monkey Pond). Basin 3 drains to Basin 4 which drains to Basin 8. Basins 6,7,8, and 9 drain directly to Basin 10. The drainage from Basin 1 to 2 to 5 to 9 represents the flow of Reservoir Brook and cumulatively contributes the largest water and nutrient load to Monkey Pond.

Septic Data

 Septic data was compiled for the LLRM based on mathematical models which calculate output of septic systems based on population data, estimated system age and degree of seasonal use. Only parcels located within 250 feet of a stream and known to have septic systems were included in the calculation. All parcels were determined to be year-round; age of the systems was undetermined, so for purposes of the model, it was assumed all systems were old rather than new. A total of 11 septic systems were identified for a total estimated load of 3.2 kg TP per year.

Catchment	# Septic Systems (Year Round)	Phosphorus Load (kg)
Basin 1	0	0
Basin 2	0	0
Basin 3	0	0
Basin 4	0	0
Basin 5	1	0.3
Basin 6	7	2.0
Basin 7	0	0
Basin 8	2	0.6
Basin 9	1	0.3
Basin 10	0	0
Totals	11	3.2

Table 1. Septic System data for the Reservoir Brook catchments.

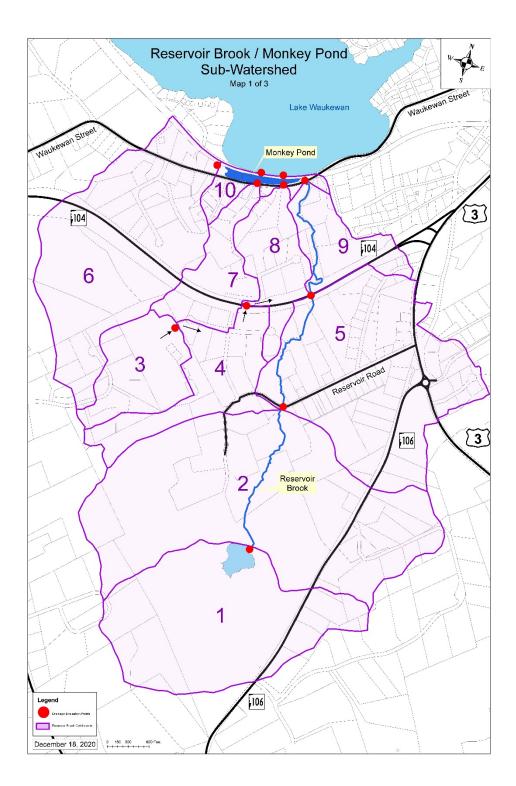


Figure 1: Reservoir Brook catchments.

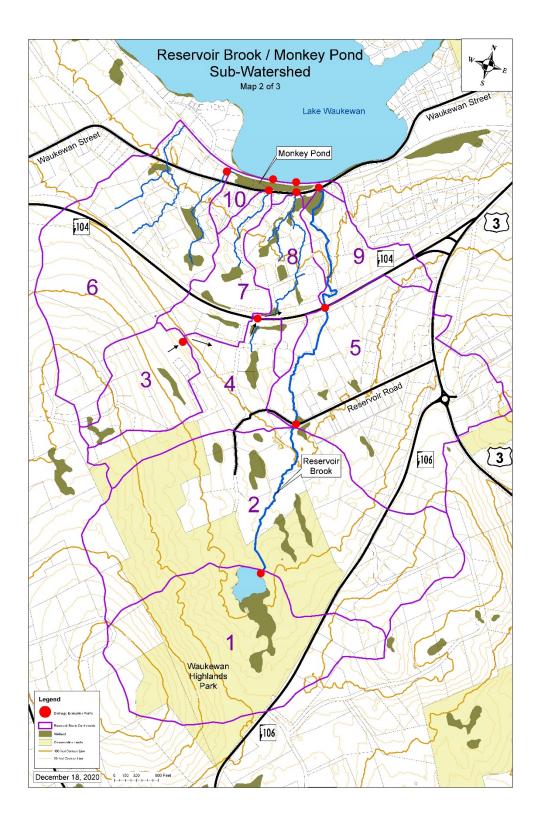


Figure 2. Map of Reservoir Brook catchments including streams parcels, roads, and 20 ft contours

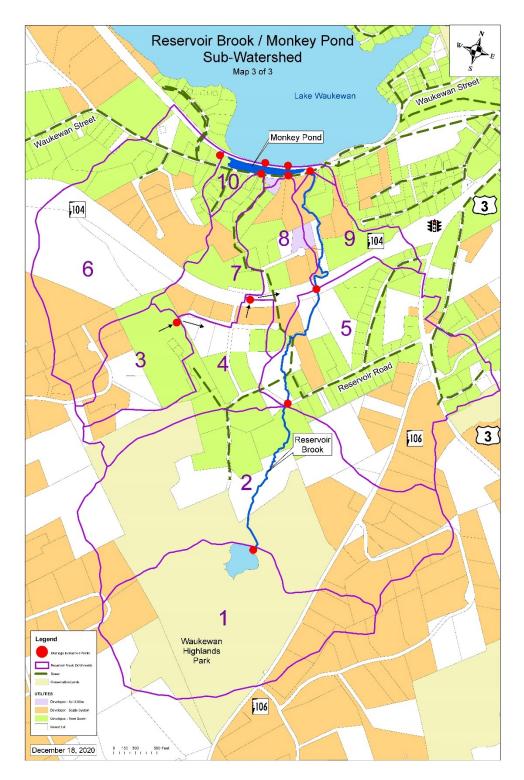


Figure 3: Reservoir Brook watershed showing parcels on town sewer and parcels with septic systems.

Table 2: Water Routing Pattern for the Reservoir Brook Watershed.											
1=YES 0=NO	BASIN 1	BASIN 2	BASIN 3	BASIN 4	BASIN 5	BASIN 6	BASIN 7	BASIN 8	BASIN 9	BASIN 10	
XXX=BLANK						Direct	Direct	Direct	Direct		
INDIVIDUAL											
BASIN	1	1	1	1	1	1	1	1	1	1	
BASIN 1 OUTPUT	XXX	1	0	0	0	0	0	0	0	0	
BASIN 2 OUTPUT	0	XXX	0	0	1	0	0	0	0	0	
BASIN 3 OUTPUT	0	0	XXX	1	0	0	0	0	0	0	
BASIN 4 OUTPUT	0	0	0	XXX	0	0	0	1	0	0	
BASIN 5 OUTPUT	0	0	0	0	XXX	0	0	0	1	0	
BASIN 6 OUTPUT	0	0	0	0	0	XXX	0	0	0	1	
BASIN 7 OUTPUT	0	0	0	0	0	0	XXX	0	0	1	
BASIN 8 OUTPUT	0	0	0	0	0	0	0	XXX	0	1	
BASIN 9 OUTPUT	0	0	0	0	0	0	0	0	XXX	1	
BASIN 10 OUTPUT	0	0	0	0	0	0	0	0	0	XXX	

Land Cover

Land cover data is used to estimate the transport and retention patterns of phosphorus as it moves through a watershed. An overlay of soil information from NRCS Web Soil Survey and Land cover data (NLCD 2011) for the study area has been verified and edited as necessary using aerial imagery by DuBois & King, Inc.

The land cover acreages for each basin are shown in Table 3, and Figure 4 shows the land cover and soils for the watershed.

LAND USE	BASIN 1	BASIN 2	BASIN 3	BASIN 4	BASIN 5	BASIN 6	BASIN 7	BASIN 8	BASIN 9	BASIN 10	Total
	AREA (HA)										
Urban 1 (Low Density Residential)	2.3	5.7	1.3	0.0	9.4	4.8	0.0	0.0	0.3	1.8	25.6
Urban 2 (Mid Density Residential/Commercial)	0.0	0.0	1.0	5.1	8.0	0.0	5.1	5.5	2.8	0.0	27.5
Urban 5 (Mowed Fields)	3.4	3.4	0.0	0.0	5.2	0.7	0.0	0.0	0.7	0.0	13.4
Agric 3 (Grazing)	0.0	0.0	1.0	0.0	0.0	2.8	0.0	0.0	0.0	0.0	3.9
Forest 3 (Mixed)	42.8	72.7	12.5	14.2	34.2	44.6	7.9	7.9	8.1	2.9	247.8
Open 1 (Wetland/Lake)	1.3	0.4	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.3	2.1
TOTAL	49.8	82.2	15.8	19.3	56.8	52.9	13.0	13.4	11.9	5.1	320.2

Table 3. Reservoir Brook Land Cover

LOAD ROUTING AND ATTENUATION: PHOSPHORUS

	BASIN 1 (KG/YR)	BASIN 2 (KG/YR)	BASIN 3 (KG/YR)	BASIN 4 (KG/YR)	BASIN 5 (KG/YR)	BASIN 6 (KG/YR)	BASIN 7 (KG/YR)	BASIN 8 (KG/YR)	BASIN 9 (KG/YR)	BASIN 10 (KG/YR)
BASIN 1 INDIVIDUAL	4.5	7.0	2.7	5.1	15.6	6.3	4.9	5.3	3.4	0.9
BASIN 1 OUTPUT	0.0	0.8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
BASIN 2 OUTPUT	0.0	0.0	0.0	0.0	5.8	0.0	0.0	0.0	0.0	0.0
BASIN 3 OUTPUT	0.0	0.0	0.0	2.0	0.0	0.0	0.0	0.0	0.0	0.0
BASIN 4 OUTPUT	0.0	0.0	0.0	0.0	0.0	0.0	0.0	5.7	0.0	0.0
BASIN 5 OUTPUT	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	17.2	0.0
BASIN 6 OUTPUT	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	5.0
BASIN 7	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	4.2
BASIN 8	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	9.9
BASIN 9	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	18.5
BASIN 10	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
CUMULATIVE TOTAL	4.5	7.8	2.7	7.2	21.5	6.3	4.9	11.0	20.5	38.5
BASIN ATTENUATION - how much passes thru	0.65	0.75	0.75	0.80	0.80	0.80	0.85	0.90	0.90	0.80
OUTPUT LOAD	3.0	5.8	2.0	5.7	17.2	5.0	4.2	9.9	18.5	30.8

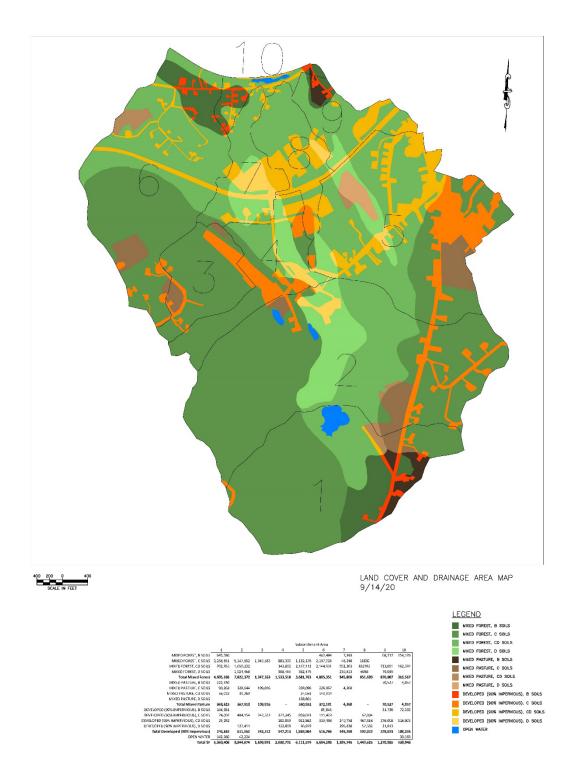


Figure 4. Land Cover and Soils Map for Reservoir Brook watershed.

.

LLRM Results

Modeling results indicate that the majority of the phosphorus contribution to Monkey Pond is from the main drainage of Reservoir Brook beginning with Basin 1, which contains Reservoir Pond (Table 3). The cumulative total phosphorus (TP) load from Basin 9 is 18.5 kg per year. This represents approximately half (48%) of the total load to Monkey Pond. The second highest load, 9.9 kg TP, comes from Basin 8, which includes the loading from Basins 3 and 4.

Of all the basins, Basin 5 contributes the highest individual TP load, 15.6 kg, due to it having the most developed land of all the basins at 17.4 hectares.

The contribution of phosphorus from septic systems appears to be quite small; 11 parcels were identified within 250 feet of the streams entering Monkey Pond, for an estimated phosphorus load of 3.2 kg/ year.

	Reserv	oir Brook Subwa	tershed
Drainage Routing	Phosphorus (kg/yr)	Percent TP Contribution	Water Load (m3/yr)
Basin 1	3.0	7.8	310,372
Basin 2	5.8	15.1	514,407
Basin 5	17.2	49.6	794,332
Basin 9 (Cumulative)	18.5	48.1	751,490
Basin 3	2.0	5.2	99,415
Basin 4	5.7	14.8	208,811
Basin 8 (Cumulative)	9.9	25.7	264,826
Basin 6 Direct	5.0	13.0	330,859
Basin 7 Direct	4.2	10.9	84,731
Basin 10 Direct	0.9	2.3	35,996
Total	38.5	100%	
Attenuation Factor	.80		.85
Total Load to Monkey Pond)	30.8		1,247,716

Table 3. Summary of total phosphorus (TP) loading by subdrainage for Monkey Pond

Appendix C



MEMORANDUM

226315

TO: Patricia Tarpey, Lake Winnipesaukee Association; John Edgar, Town of Meredith FROM: Nicholas Sceggell, PE

SUBJECT: Reservoir Brook & Monkey Pond, Meredith, NH; Hydrologic & Hydraulic Evaluation

- DATE: December 28, 2020
 - 1. The Lake Winnipesaukee Association has requested that DuBois & King, Inc. assist with a study of the Reservoir Brook and Monkey Pond subwatershed, in an effort to better understand the processes of excessive stormwater loading from land development into Lake Waukewan and further downstream. A part of this study is a drainage analysis and hydrologic assessment of the Reservoir Brook/Monkey Pond Watershed.

The watershed was split into subcatchments to evaluate locations in the watershed of interest, specifically looking at culverts along Reservoir Brook at Reservoir Road, NH Route 104, and on Waukewan Street. Two additional culverts under Waukewan Street that discharge into Monkey Pond were evaluated, as well as an area that drains along the railroad tracks from the west into Monkey Pond. The Meredith Reservoir, the headwaters for Reservoir Brook, was also included in the analysis.

- 2. The Town of Meredith provided subcatchment mapping of the watershed. The mapping was produced using LiDAR topography, and through field verification of drainage routing throughout the watershed. The town prepared GIS shapefiles of each subcatchment. The shapefiles were imported into AutoCAD software with an aerial overlay.
- 3. Land uses within each of the subcatchments were categorized in order to assign stormwater runoff characteristics. The initial land cover information was imported from NH GRANIT. Using updated aerial photography and ground-truthing to check questionable land use areas, the land cover information was updated using AutoCAD software. Subwatershed Maps are included in Appendix A.
- 4. Soil information was imported to overlay and further define the land cover by hydrologic soil grouping. Soils data from the Natural Resources Conservation Service (NRCS) web soil survey was used for this evaluation. A copy of the soils report is attached as Appendix B.
- 5. Rainfall data used for the evaluation was downloaded from the Northeast

Regional Climate Center. A copy of the precipitation table is attached as Appendix C.

6. HydroCAD water modeling software was used to evaluate the points of interest for each of the subwatershed areas. HydroCAD utilizes methodology developed through the NRCS TR-20 unit hydrograph routing model, and allows graphical representation of the watershed and subwatershed routing. Modeling of the watershed was done for various storm frequencies in order to identify the peak discharge rates that occur as a result of the 24-hour duration storms with a return frequency of once in 2 years, 10 years, 25 years, 50 years, and 100 years. Table 1 summarizes the peak discharges for each subwatershed and storm frequency.

	2 Yr	10 Yr	25 Yr	50 Yr	100 Yr
Subcatchment	Rainfall	Rainfall	Rainfall	Rainfall	Rainfall
Area	Runoff	Runoff	Runoff	Runoff	Runoff
1S	39.23	101.84	157.57	213.12	281.01
2S	70.80	165.09	246.12	326.01	422.59
3S	18.28	41.30	61.11	80.49	103.78
4S	29.18	59.40	84.44	108.56	137.17
5S	84.16	167.98	236.68	302.08	379.49
6S	38.50	93.01	140.56	187.25	243.81
7S	24.40	45.73	62.80	78.85	97.70
8S	24.82	46.59	64.02	80.41	99.67
9S	16.61	34.68	49.70	64.14	81.41
10S	9.18	19.75	28.63	37.19	47.42

Table 1: Peak Runoff in Reservoir Brook Subwatersheds

Runoff units are cubic feet per second (CFS)

7. A summary of the water routing can be found in the Summary Report on the mapping of existing conditions and nutrient modeling for Reservoir Brook Table 2, prepared by Pat Tarpey, Lake Winnipesaukee Association. A copy of Table 2 is attached below.

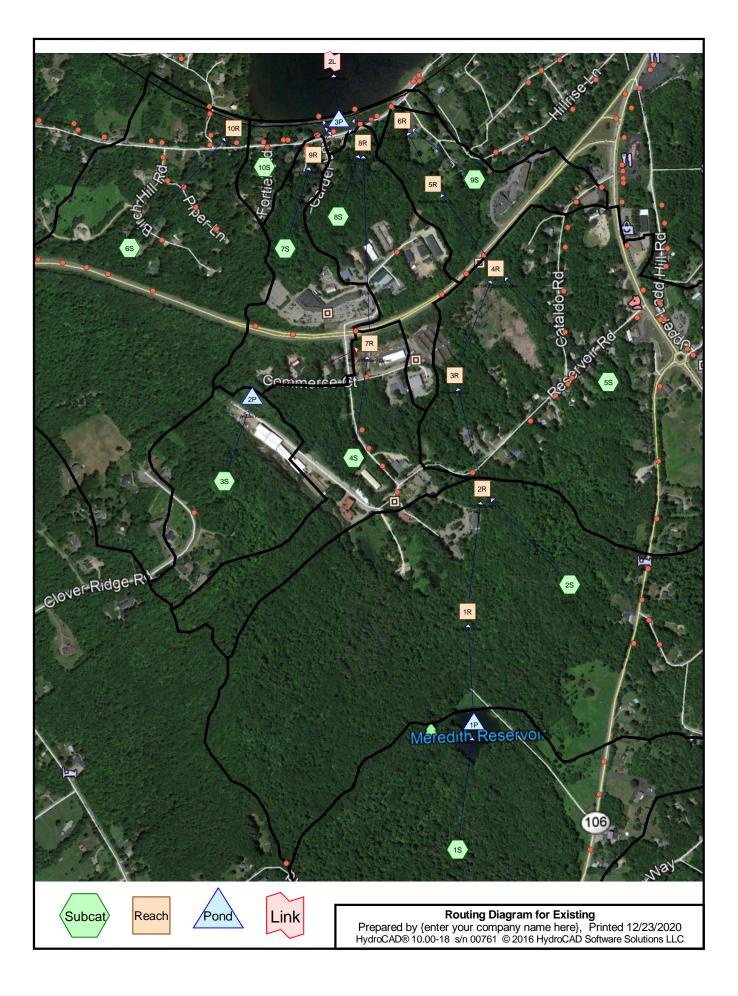
1=YES 0=NO XXX=BLANK	BASIN 1	BASIN 2	BASIN 3	BASIN 4	BASIN 5	BASIN 6 Direct	BASIN 7 Direct	BASIN 8 Direct	BASIN 9 Direct	BASIN 10
INDIVIDUAL BASIN	1	1	1	1	1	1	1	1	1	1
BASIN 1 OUTPUT	XXX	1	0	0	0	0	0	0	0	0
BASIN 2 OUTPUT	0	XXX	0	0	1	0	0	0	0	0
BASIN 3 OUTPUT	0	0	XXX	1	0	0	0	0	0	0
BASIN 4 OUTPUT	0	0	0	XXX	0	0	0	1	0	0
BASIN 5 OUTPUT	0	0	0	0	XXX	0	0	0	1	0
BASIN 6 OUTPUT	0	0	0	0	0	XXX	0	0	0	1
BASIN 7 OUTPUT	0	0	0	0	0	0	XXX	0	0	1
BASIN 8 OUTPUT	0	0	0	0	0	0	0	XXX	0	1
BASIN 9 OUTPUT	0	0	0	0	0	0	0	0	XXX	1
BASIN 10 OUTPUT	0	0	0	0	0	0	0	0	0	XXX

- 8. Seven culverts were evaluated comparing the estimated hydraulic capacity of the culvert to the estimated peak discharge from the HydroCAD model. Capacity for each culvert was estimated using HydroCAD modeling software and physical characteristics of the pipe, including size, material of construction, and approximate slope. Information was gathered from the NH Statewide Asset Data Exchange System (SADES) and field work. Results are presented in Table 3: Culvert Evaluation.
- 9. The Meredith Reservoir is the headworks of Reservoir Brook and was incorporated into the hydraulic model as a pond with some limited storage capacity. The size of the low flow outlet of the pond was estimated at 18" diameter but could not be confirmed because the outlet was blocked with leaves and debris. The secondary outlet consists of three (3) 36" smooth plastic culverts. The pond is able to store and discharge all storm events evaluated without overtopping the dam embankment. Discharge flows from the dam were routed downstream along the Reservoir Brook channel for this evaluation. Also incorporated into the model was the constructed stormwater pond on Tax Map S24, Lot 17, to best represent flows impacted by the pond.
- 10. The area draining to the pond from the West was routed through an open channel into the pond which simulates the drainage ditch along the south side of the railroad which directs water into Monkey Pond. The hydraulic model indicates that the ditch has capacity to carry the 25 year storm event but overtops for larger storm events.
- 11. Field review of Reservoir Brook was completed to accurately model the stream reach in the hydraulic model. Preliminary observations of the stream's geomorphology indicate a stream that is moderately entrenched with a stream slope of 2-4%, and moderate sinuosity. A more comprehensive geomorphic analysis of the stream was not part of this project scope. Any culvert or stream restoration work would include a more detailed review of the stream and reference reaches adjacent to proposed work.

12. NH Stream Crossing Guidelines published by University of NH were utilized to evaluate the Reservoir Brook culverts at Reservoir Rd, Route 104, and Waukewan Street. Deficiencies were noted for organic material transport, sediment transport, aquatic organism passage, and lack of capacity for flood events.

Table 3: Culvert Evaluation

Culvert Location	Estimated Discharge Capacity	Pass 10 yr Storm	Pass 25 yr Storm	Pass 50 yr Storm	Pass 100 yr Storm	Notes
Reservoir Brook at Reservoir Road	205.67 cfs	Yes	No	No	No	48" Concrete Pipe, s=0.0147 ft/ft
Reservoir Brook at NH Route 104	1271.3 cfs	Yes	Yes	Yes	Yes	72" Concrete Pipe, s=0.0645 ft/ft
Reservoir Brook at Waukewan St. (Eastern Culvert into Monkey Pond)	30.24 cfs	No	No	No	No	48" Corrugated Metal, s=0.0016 ft/ft, pipe is sediment filled to 50%.
Waukewan St - Central Culvert into Monkey Pond	9.59 cfs	No	No	No	No	18" Plastic, s=0.0083 ft/ft
Waukewan St - Western Culvert into Monkey Pond	28.33 cfs	No	No	No	No	18" Plastic, s=0.0727 ft/ft
Eastern Railroad Culvert - Monkey Pond into Lake Waukewan	67.23 cfs	No	No	No	No	48" Steel Pipe, s=0.020 ft/ft, Lake Waukewan tailwater during normal Iake levels
Western Railroad Culvert - Monkey Pond into Lake Waukewan	85.92 cfs	No	No	No	No	48" Concrete Pipe, s=0.0147 ft/ft, Lake Waukewan tailwater during normal lake levels



Area Listing (all nodes)

Area	CN	Description
(acres)		(subcatchment-numbers)
6.030	79	50-75% Grass cover, Fair, HSG C (1S, 2S, 5S, 6S)
6.830	61	>75% Grass cover, Good, HSG B (1S, 9S, 10S)
25.240	74	>75% Grass cover, Good, HSG C (1S, 2S, 3S, 5S, 6S, 7S)
4.560	80	>75% Grass cover, Good, HSG D (5S)
7.670	92	Urban commercial, 85% imp, HSG B (1S, 6S, 9S, 10S)
106.270	94	Urban commercial, 85% imp, HSG C (1S, 2S, 3S, 4S, 5S, 6S, 7S, 8S, 9S, 10S)
16.580	95	Urban commercial, 85% imp, HSG D (2S, 4S, 5S, 7S, 8S, 9S)
4.940	98	Water Surface, 0% imp, HSG A (1S, 2S, 10S)
213.030	73	Woods, Fair, HSG C (1S, 2S, 4S, 5S, 6S, 7S, 8S, 9S, 10S)
30.370	55	Woods, Good, HSG B (1S, 6S, 7S, 9S, 10S)
321.880	70	Woods, Good, HSG C (1S, 2S, 3S, 4S, 5S, 6S, 7S, 8S)
46.860	77	Woods, Good, HSG D (2S, 4S, 5S, 7S, 8S, 9S)
790.260	75	TOTAL AREA

Soil Listing (all nodes)

Area	Soil	Subcatchment
(acres)	Group	Numbers
4.940	HSG A	1S, 2S, 10S
44.870	HSG B	1S, 6S, 7S, 9S, 10S
672.450	HSG C	1S, 2S, 3S, 4S, 5S, 6S, 7S, 8S, 9S, 10S
68.000	HSG D	2S, 4S, 5S, 7S, 8S, 9S
0.000	Other	
790.260		TOTAL AREA

Ground Covers (all nodes)

HSG-/	A HSG-B	HSG-C	HSG-D	Other	Total	Ground	Subcatchment
(acres	s) (acres)	(acres)	(acres)	(acres)	(acres)	Cover	Numbers
0.00	0.000	6.030	0.000	0.000	6.030	50-75% Grass cover, Fair	1S, 2S,
							5S, 6S
0.00	0 6.830	25.240	4.560	0.000	36.630	>75% Grass cover, Good	1S, 2S,
							3S, 5S,
							6S, 7S,
							9S, 10S
0.00	0 7.670	106.270	16.580	0.000	130.520	Urban commercial, 85% imp	1S, 2S,
							3S, 4S,
							5S, 6S,
							7S, 8S,
							9S, 10S
4.94	0.000	0.000	0.000	0.000	4.940	Water Surface, 0% imp	1S, 2S,
							10S
0.00	0.000	213.030	0.000	0.000	213.030	Woods, Fair	1S, 2S,
							4S, 5S,
							6S, 7S,
							8S, 9S,
							10S
0.00	0 30.370	321.880	46.860	0.000	399.110	Woods, Good	1S, 2S,
							3S, 4S,
							5S, 6S,
							7S, 8S,
		070 450					9S, 10S
4.94	0 44.870	672.450	68.000	0.000	790.260	TOTAL AREA	

Existing

Line#	Node Number	In-Invert (feet)	Out-Invert (feet)	Length (feet)	Slope (ft/ft)	n	Diam/Width (inches)	Height (inches)	Inside-Fill (inches)
1	2R	682.31	681.40	62.0	0.0147	0.011	48.0	0.0	0.0
2	4R	622.00	614.00	124.0	0.0645	0.011	72.0	0.0	0.0
3	6R	542.00	541.90	61.0	0.0016	0.025	54.0	0.0	18.0
4	7R	640.00	636.00	120.0	0.0333	0.013	36.0	0.0	0.0
5	8R	542.50	542.00	60.0	0.0083	0.013	16.0	0.0	0.0
6	9R	544.00	540.00	55.0	0.0727	0.013	18.0	0.0	0.0
7	1P	778.00	777.40	30.0	0.0200	0.013	30.0	0.0	0.0
8	3P	541.00	540.20	40.0	0.0200	0.011	48.0	0.0	0.0
9	3P	541.00	540.00	47.0	0.0213	0.012	48.0	0.0	0.0

Pipe Listing (all nodes)

Existing
Prepared by {enter your company name here}
HvdroCAD® 10.00-18 s/n 00761 © 2016 HvdroCAD Software Solutions LL

Time span=2.00-48.00 hrs, dt=0.01 hrs, 4601 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

C

Subcatchment 1S: Subcat 1Runoff Area=123.050 ac3.86% ImperviousRunoff Depth=0.56"Flow Length=2,832'Slope=0.1200 '/'Tc=28.2 minCN=70Runoff=39.23 cfs5.787 af
Subcatchment 2S: Subcat 2Runoff Area=203.010 ac 5.87% Impervious Runoff Depth=0.69"Flow Length=4,382'Slope=0.0950 '/' Tc=41.3 min CN=73 Runoff=70.80 cfs 11.682 af
Subcatchment 3S: Subcat 3Runoff Area=39.030 ac12.13% ImperviousRunoff Depth=0.74"Flow Length=2,511'Slope=0.0910 '/'Tc=26.3 minCN=74Runoff=18.28 cfs2.393 af
Subcatchment 4S: Subcat 4Runoff Area=47.760 ac22.35% ImperviousRunoff Depth=0.93"Flow Length=3,272'Slope=0.0980 '/'Tc=27.9 minCN=78Runoff=29.18 cfs3.717 af
Subcatchment 5S: Subcat 5Runoff Area=140.300 ac26.00% ImperviousRunoff Depth=0.99"Flow Length=3,798'Slope=0.0810 '/'Tc=33.5 minCN=79Runoff=84.16 cfs11.553 af
Subcatchment 6S: Subcat 6Runoff Area=130.360 ac7.73% ImperviousRunoff Depth=0.65"Flow Length=4,593'Slope=0.0810 '/'Tc=47.8 minCN=72Runoff=38.50 cfs7.028 af
Subcatchment 7S: Subcat 7Runoff Area=32.040 ac33.21% ImperviousRunoff Depth=1.16"Flow Length=3,154'Slope=0.0630 '/'Tc=29.8 minCN=82Runoff=24.40 cfs3.104 af
Subcatchment 8S: Subcat 8Runoff Area=33.140 ac34.86% ImperviousRunoff Depth=1.16"Flow Length=2,745'Slope=0.0470 '/'Tc=30.8 minCN=82Runoff=24.82 cfs3.210 af
Subcatchment 9S: Subcat 9Runoff Area=29.180 ac22.05% ImperviousRunoff Depth=0.88"Flow Length=2,576'Slope=0.0700 '/'Tc=28.0 minCN=77Runoff=16.61 cfs2.144 af
Subcatchment 10S: Subcat 10Runoff Area=12.390 ac 29.64% Impervious Runoff Depth=0.83"Flow Length=961'Slope=0.0830 '/' Tc=12.1 min CN=76Runoff=9.18 cfs 0.858 af
Reach 1R: Reservoir Brook Avg. Flow Depth=0.42' Max Vel=3.79 fps Inflow=8.21 cfs 5.570 af n=0.040 L=2,356.0' S=0.0398 '/' Capacity=202.98 cfs Outflow=8.20 cfs 5.560 af
Reach 2R: Res Rd Culvert Avg. Flow Depth=1.62' Max Vel=14.86 fps Inflow=70.93 cfs 17.243 af 48.0" Round Pipe n=0.011 L=62.0' S=0.0147 '/' Capacity=205.67 cfs Outflow=70.92 cfs 17.242 af
Reach 3R: Reservoir Brook Avg. Flow Depth=1.99' Max Vel=7.06 fps Inflow=70.92 cfs 17.242 af n=0.040 L=1,983.0' S=0.0300 '/' Capacity=156.15 cfs Outflow=69.92 cfs 17.236 af
Reach 4R: Rt 104 Easterly Culvert Avg. Flow Depth=1.34' Max Vel=29.51 fps Inflow=139.01 cfs 28.788 af 72.0" Round Pipe n=0.011 L=124.0' S=0.0645 '/' Capacity=1,271.30 cfs Outflow=138.98 cfs 28.788 af
Reach 5R: Reservoir Brook Avg. Flow Depth=2.46' Max Vel=8.67 fps Inflow=138.98 cfs 28.788 af n=0.040 L=2,167.0' S=0.0332 '/' Capacity=201.12 cfs Outflow=137.39 cfs 28.781 af
Reach 6R: Easterly Culvert into PondAvg. Flow Depth=3.00'Max Vel=2.49 fpsInflow=147.84 cfs30.925 af54.0" Round Pipe w/ 18.0" inside filln=0.025L=61.0'S=0.0016 '/'Capacity=24.84 cfsOutflow=25.57 cfs30.924 af

Existing Prepared by {enter your company name here} HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLC	Type III 24-hr 2yr Rainfall=2.72" Printed 12/23/2020 Page 7								
Reach 7R: Rt 104 Westerly Culvert Avg. Flow Depth=1.00' Max Vel=14.15 fps Inflow=29.18 cfs 4.571 af 36.0" Round Pipe n=0.013 L=120.0' S=0.0333 '/' Capacity=121.77 cfs Outflow=29.16 cfs 4.571 af									
Reach 8R: Central Culvert into Pond Avg. Flow Depth=1.33' Max Vel=5.72 fps Inflow=53.99 cfs 7.781 a 16.0" Round Pipe n=0.013 L=60.0' S=0.0083 '/' Capacity=7.00 cfs Outflow=7.30 cfs 7.781 a									
Reach 9R: Westerly culvert into PondAvg. Flow Depth=1.07'Max18.0"Round Pipen=0.013L=55.0'S=0.0727 '/'Capacit									
Reach 10R: (new Reach) Avg. Flow Depth=1.31' Max n=0.040 L=116.0' S=0.0172 '/' Capacit	x Vel=4.22 fps Inflow=38.50 cfs 7.028 af ty=96.39 cfs Outflow=38.49 cfs 7.028 af								
Pond 1P: Meredith Reservoir Peak Elev=778.64' Stor 30.0" Round Culvert x 3.00 n=0.013 L=30.0'	rage=2.199 af Inflow=39.23 cfs 5.787 af S=0.0200 '/' Outflow=8.21 cfs 5.570 af								
Pond 2P: Wilcom PondPeak Elev=715.93' StorPrimary=1.99 cfs0.854 afSecondary=0.00	rage=1.603 af Inflow=18.28 cfs 2.393 af cfs 0.000 af Outflow=1.99 cfs 0.854 af								
Pond 3P: Monkey PondPeak Elev=543.22' Stora	age=4.388 af Inflow=90.96 cfs 49.695 af Outflow=68.42 cfs 48.557 af								
Link 2L: Lake Waukewan	Inflow=68.42 cfs 48.557 af Primary=68.42 cfs 48.557 af								

Total Runoff Area = 790.260 ac Runoff Volume = 51.476 af Average Runoff Depth = 0.78" 85.96% Pervious = 679.318 ac 14.04% Impervious = 110.942 ac

Summary for Subcatchment 1S: Subcat 1

Runoff = 39.23 cfs @ 12.47 hrs, Volume= 5.787 af, Depth= 0.56"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2yr Rainfall=2.72"

Area	(ac)	CN	Desc	Description						
14.	820	55	Woo	/oods, Good, HSG B						
74.	760	70	Woo	ds, Good,	HSG C					
16.	140	73	Woo	ds, Fair, H	ISG C					
5.	100	61	>75%	6 Grass co	over, Good,	HSG B				
2.	070	74	>75%	6 Grass co	over, Good,	HSG C				
1.	290	79	50-75	5% Grass	cover, Fair	, HSG C				
3.	310	92	Urba	n commei	cial, 85% iı	mp, HSG B				
1.	700	94				mp, HSG C				
0.	580	94	Urba	n commei	cial, 85% iı	mp, HSG C				
3.	280	98	Wate	er Surface	<u>, 0% imp, F</u>	ISG A				
123.	050	70	Weig	hted Aver	age					
118.	298		96.14	1% Pervio	us Area					
4.	751		3.86%	% Impervi	ous Area					
Tc	Length		Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
28.2	2,832	2 0.	1200	1.68		Lag/CN Method,				

Summary for Subcatchment 2S: Subcat 2

Runoff = 70.80 cfs @ 12.63 hrs, Volume= 11.682 af, Depth= 0.69"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2yr Rainfall=2.72"

Area (ac)	CN	Description
118.170	70	Woods, Good, HSG C
37.880	73	Woods, Fair, HSG C
23.520	77	Woods, Good, HSG D
7.540	74	>75% Grass cover, Good, HSG C
0.900	79	50-75% Grass cover, Fair, HSG C
11.110	94	Urban commercial, 85% imp, HSG C
2.920	95	Urban commercial, 85% imp, HSG D
0.970	98	Water Surface, 0% imp, HSG A
203.010	73	Weighted Average
191.085		94.13% Pervious Area
11.925		5.87% Impervious Area

Existing	
Prepared by {enter your company name here}	
HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solution	is LLC

	Тс	Length	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	-	
	41.3	4,382	0.0950	1.77		Lag/CN Method,	

Summary for Subcatchment 3S: Subcat 3

Runoff = 18.28 cfs @ 12.42 hrs, Volume= 2.393 af, Depth= 0.74"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2yr Rainfall=2.72"

_	Area	(ac)	CN	Desc	cription		
	30.	940	70	Woo	ds, Good,	HSG C	
	2.	520	74	>75%	% Grass co	over, Good,	I, HSG C
_	5.	570	94	Urba	in commei	rcial, 85% ii	imp, HSG C
	39.	030	74	Weig	ghted Aver	age	
	34.	295		87.8	7% Pervio	us Area	
	4.	734		12.13	3% Imperv	ious Area	
	-			. .		•	
	Tc	Lengt		Slope	Velocity	Capacity	Description
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
	26.3	2,51	1 0	.0910	1.59		Lag/CN Method,
							-

Summary for Subcatchment 4S: Subcat 4

Runoff =	29.18 cfs @	12.43 hrs, Volume=	3.717 af, Depth= 0.93"
----------	-------------	--------------------	------------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2yr Rainfall=2.72"

Area (a	ac) C	N Des	scription			
20.23	30 7	70 Wo	ods, Good,	HSG C		
7.89	90 7	'3 Wo	ods, Fair, F	ISG C		
7.08	80 7	7 Wo	ods, Good,	HSG D		
7.40	00 9	94 Urb	an comme	rcial, 85% iı	mp, HSG C	
2.34	40 9	94 Urb	an comme	rcial, 85% iı	mp, HSG C	
2.82	20 9	95 Urb	an comme	rcial, 85% iı	mp, HSG D	
47.70	60 7	′8 We	ighted Ave	rage		
37.08	84	77.0	55% Pervio	us Area		
10.6	76	22.3	35% Imperv	vious Area		
Tc l	Length	Slope		Capacity	Description	
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)		
27.9	3,272	0.0980	1.96		Lag/CN Method,	

Summary for Subcatchment 5S: Subcat 5

Runoff = 84.16 cfs @ 12.50 hrs, Volume= 11.553 af, Depth= 0.99"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2yr Rainfall=2.72"

Area (ac)	CN	Descript	Description						
25.990	70	Woods,	Noods, Good, HSG C						
49.750	73	Woods,	Fair, HS	GC					
8.780	77	Woods,	Good, H	SG D					
7.760	74	>75% G	rass cov	er, Good,	HSG C				
0.550	79	50-75%	Grass co	over, Fair	, HSG C				
4.560	80	>75% G	rass cov	er, Good,	HSG D				
19.720	94	Urban co	ommerci	al, 85% ir	np, HSG C				
20.960	94	Urban co	ommerci	al, 85% ir	np, HSG C				
2.230	95	Urban co	ommerci	al, 85% ir	np, HSG D				
140.300	79	Weighte	ed Averag	ge					
103.826		74.00%	Pervious	Area					
36.473		26.00%	Impervio	ous Area					
Tc Len	gth	Slope Ve	elocity C	Capacity	Description				
(min) (fe	et)	(ft/ft) (ft	t/sec)	(cfs)					
33.5 3,7	798 0	.0810	1.89		Lag/CN Method,				

Summary for Subcatchment 6S: Subcat 6

Runoff = 38.50 cfs @ 12.75 hrs, Volume= 7.028 af, Depth= 0.65"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2yr Rainfall=2.72"

Area	(ac)	CN	Desc	ription				
10.	260	55	Wood	ds, Good,	HSG B			
50.	460	70	Woods, Good, HSG C					
49.	240	73	Wood	Woods, Fair, HSG C				
5.250 74			>75% Grass cover, Good, HSG C					
3.	290	79 50-75% Grass cover, Fair, HSG C						
1.970 92 Urban co				n commei	mmercial, 85% imp, HSG B			
2.	560	94	Urba	n commei	rcial, 85% ii	mp, HSG C		
7.330		94	Urban commercial, 85% imp, HSG C					
130.360		72	Weighted Average					
120.279			92.27% Pervious Area					
10.081			7.73% Impervious Area					
				-				
Тс	Length	n 8	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
47.8	4,593	3 0.	.0810	1.60		Lag/CN Method,		
47.8	4,593	s 0.	.0810	1.60		Lag/UN Method,		

Summary for Subcatchment 7S: Subcat 7

Runoff = 24.40 cfs @ 12.42 hrs, Volume= 3.104 af, Depth= 1.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2yr Rainfall=2.72"

	Area (a	ic)	CN	Desc	ription			
	0.1	70	55	Woo	ds, Good,	HSG B		
	1.0	60	70	Woo	ds, Good,	HSG C		
	12.6	80	73	Woo	ds, Fair, H	ISG C		
	5.5	10	77	Woo	ds, Good,	HSG D		
	0.1	00	74	>75%	6 Grass co	over, Good,	HSG C	
	5.7	30	94	Urba	n commer	cial, 85% ir	mp, HSG C	
	6.7	90	95	Urba	n commer	cial, 85% ir	mp, HSG D	
	32.0	40	82	Weig	hted Aver	age		
	21.3	98		66.79	9% Pervio	us Area		
	10.6	42		33.21	I% Imperv	vious Area		
	Tc I	_ength	n 8	Slope	Velocity	Capacity	Description	
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
	29.8	3,154	0.	.0630	1.77		Lag/CN Method,	

Summary for Subcatchment 8S: Subcat 8

Runoff = 24.82 cfs @ 12.45 hrs, Volume= 3.210 af, Depth= 1.1	6"
--	----

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2yr Rainfall=2.72"

Area	(ac) (CN	Desc	ription			
0.	270	70	Woo	ds, Good,	HSG C		
19.	120	73	Woo	ds, Fair, H	ISG C		
0.	160	77	Woo	ds, Good,	HSG D		
1.	540	94	Urba	n commei	rcial, 85% ir	mp, HSG C	
10.	730	94	Urba	n commei	rcial, 85% ir	mp, HSG C	
1.3	320	95	Urba	n commei	rcial, 85% ir	mp, HSG D	
33.	140	82	Weig	hted Aver	age		
21.	588		65.14	4% Pervio	us Area		
11.	551		34.86	5% Imperv	vious Area		
Тс	Length		Slope	Velocity	Capacity	Description	
(min)	(feet)		(ft/ft)	(ft/sec)	(cfs)	Description	
30.8	. ,		. /	1.48	(013)	Lag/CN Method,	
30.8	2,745	0.0	0470	1.40		Lay CN Method,	

Summary for Subcatchment 9S: Subcat 9

Runoff = 16.61 cfs @ 12.42 hrs, Volume= 2.144 af, Depth= 0.88"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2yr Rainfall=2.72"

 Area (ad	c) (N	Desc	ription			
1.58	80 8	55	Woo	ds, Good,	HSG B		
16.60	0	73	Woo	ds, Fair, H	ISG C		
1.81	0	77	Woo	ds, Good,	HSG D		
1.62	20 (61	>75%	Grass co	over, Good,	HSG B	
0.73	SO 9	92	Urba	n commer	rcial, 85% ii	mp, HSG B	
6.34	0 9	94	Urba	n commer	rcial, 85% ii	mp, HSG C	
 0.50	0 9	95	Urba	n commer	rcial, 85% ii	mp, HSG D	
29.18	SO 7	77	Weig	hted Aver	age		
22.74	5		77.95	% Pervio	us Area		
6.43	5		22.05	5% Imperv	vious Area		
				•			
Tc L	ength	S	lope	Velocity	Capacity	Description	
 (min)	(feet)		(ft/ft)	(ft/sec)	(cfs)		
 28.0	2,576	0.0	0700	1.53		Lag/CN Method,	
						-	

Summary for Subcatchment 10S: Subcat 10

Runoff	=	9.18 cfs @	12.18 hrs, Volume=	0.858 af, Depth= 0.83"
--------	---	------------	--------------------	------------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 2yr Rainfall=2.72"

Area (a	c) C	N Des	cription			
3.54	40 5	5 Woo	ods, Good,	HSG B		
3.73	30 7	'3 Woo	ods, Fair, ⊢	ISG C		
0.110 61 >75% Grass cover, Good, HSG B						
1.66	50 9	2 Urba	an commei	rcial, 85% iı	mp, HSG B	
2.66	50 9	4 Urba	an commei	rcial, 85% iı	mp, HSG C	
0.69	90 9	8 Wat	er Surface	, 0% imp, H	ISG A	
12.39	90 7	'6 Wei	ghted Aver	age		
8.71	18	70.3	6% Pervio	us Area		
3.67	72	29.6	4% Imper	/ious Area		
Tc L	ength	Slope	Velocity	Capacity	Description	
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)		
12.1	961	0.0830	1.33		Lag/CN Method,	

Summary for Reach 1R: Reservoir Brook

 Inflow Area =
 123.050 ac,
 3.86% Impervious, Inflow Depth >
 0.54" for 2yr event

 Inflow =
 8.21 cfs @
 13.99 hrs, Volume=
 5.570 af

 Outflow =
 8.20 cfs @
 14.29 hrs, Volume=
 5.560 af, Atten= 0%, Lag= 17.6 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 3.79 fps, Min. Travel Time= 10.4 min Avg. Velocity = 1.77 fps, Avg. Travel Time= 22.1 min

Peak Storage= 5,103 cf @ 14.11 hrs Average Depth at Peak Storage= 0.42' Bank-Full Depth= 3.00' Flow Area= 19.5 sf, Capacity= 202.98 cfs

5.00' x 3.00' deep channel, n= 0.040 Mountain streams Side Slope Z-value= 0.5 '/' Top Width= 8.00' Length= 2,356.0' Slope= 0.0398 '/' Inlet Invert= 776.00', Outlet Invert= 682.31'



Summary for Reach 2R: Res Rd Culvert

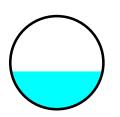
[52] Hint: Inlet/Outlet conditions not evaluated[62] Hint: Exceeded Reach 1R OUTLET depth by 1.51' @ 12.57 hrs

Inflow Area =	326.060 ac,	5.11% Impervious, Inflow	Depth > 0.63"	for 2yr event
Inflow =	70.93 cfs @	12.63 hrs, Volume=	17.243 af	
Outflow =	70.92 cfs @	12.64 hrs, Volume=	17.242 af, Atte	en= 0%, Lag= 0.6 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs / 2 Max. Velocity= 14.86 fps, Min. Travel Time= 0.1 min Avg. Velocity = 5.17 fps, Avg. Travel Time= 0.2 min

Peak Storage= 296 cf @ 12.64 hrs Average Depth at Peak Storage= 1.62' Bank-Full Depth= 4.00' Flow Area= 12.6 sf, Capacity= 205.67 cfs

48.0" Round Pipe n= 0.011 Concrete pipe, straight & clean Length= 62.0' Slope= 0.0147 '/' Inlet Invert= 682.31', Outlet Invert= 681.40'



Summary for Reach 3R: Reservoir Brook

[62] Hint: Exceeded Reach 2R OUTLET depth by 0.41' @ 12.85 hrs

 Inflow Area =
 326.060 ac,
 5.11% Impervious, Inflow Depth >
 0.63"
 for 2yr event

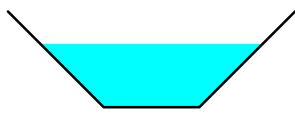
 Inflow =
 70.92 cfs @
 12.64 hrs, Volume=
 17.242 af

 Outflow =
 69.92 cfs @
 12.79 hrs, Volume=
 17.236 af, Atten= 1%, Lag= 9.0 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 7.06 fps, Min. Travel Time= 4.7 min Avg. Velocity = 2.35 fps, Avg. Travel Time= 14.1 min

Peak Storage= 19,650 cf @ 12.71 hrs Average Depth at Peak Storage= 1.99' Bank-Full Depth= 3.00' Flow Area= 18.0 sf, Capacity= 156.15 cfs

3.00' x 3.00' deep channel, n= 0.040 Mountain streams Side Slope Z-value= 1.0 '/' Top Width= 9.00' Length= 1,983.0' Slope= 0.0300 '/' Inlet Invert= 681.40', Outlet Invert= 622.00'



Summary for Reach 4R: Rt 104 Easterly Culvert

[52] Hint: Inlet/Outlet conditions not evaluated[62] Hint: Exceeded Reach 3R OUTLET depth by 0.27' @ 11.86 hrs

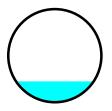
 Inflow Area =
 466.360 ac, 11.40% Impervious, Inflow Depth > 0.74" for 2yr event

 Inflow =
 139.01 cfs @ 12.62 hrs, Volume=
 28.788 af

 Outflow =
 138.98 cfs @ 12.63 hrs, Volume=
 28.788 af, Atten= 0%, Lag= 0.3 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 29.51 fps, Min. Travel Time= 0.1 min Avg. Velocity = 9.14 fps, Avg. Travel Time= 0.2 min

Peak Storage= 584 cf @ 12.63 hrs Average Depth at Peak Storage= 1.34' Bank-Full Depth= 6.00' Flow Area= 28.3 sf, Capacity= 1,271.30 cfs 72.0" Round Pipe n= 0.011 Concrete pipe, straight & clean Length= 124.0' Slope= 0.0645 '/' Inlet Invert= 622.00', Outlet Invert= 614.00'



Summary for Reach 5R: Reservoir Brook

[62] Hint: Exceeded Reach 4R OUTLET depth by 1.13' @ 12.74 hrs

Inflow Are	a =	466.360 ac, 1	1.40% Imp	ervious,	Inflow	Depth >	0.74"	for 2yr event	
Inflow	=	138.98 cfs @	12.63 hrs,	Volume	=	28.788	af		
Outflow	=	137.39 cfs @	12.76 hrs,	Volume	=	28.781	af, Atte	en= 1%, Lag= 7	7.9 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 8.67 fps, Min. Travel Time= 4.2 min Avg. Velocity = 2.52 fps, Avg. Travel Time= 14.3 min

Peak Storage= 34,348 cf @ 12.69 hrs Average Depth at Peak Storage= 2.46' Bank-Full Depth= 3.00' Flow Area= 21.0 sf, Capacity= 201.12 cfs

4.00' x 3.00' deep channel, n= 0.040 Mountain streams Side Slope Z-value= 1.0 '/' Top Width= 10.00' Length= 2,167.0' Slope= 0.0332 '/' Inlet Invert= 614.00', Outlet Invert= 542.00'



Summary for Reach 6R: Easterly Culvert into Pond

[52] Hint: Inlet/Outlet conditions not evaluated

[55] Hint: Peak inflow is 595% of Manning's capacity

[76] Warning: Detained 11.607 af (Pond w/culvert advised)

[62] Hint: Exceeded Reach 5R OUTLET depth by 4.33' @ 26.63 hrs

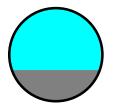
Inflow Are	ea =	495.540 ac, 12.02% Impervious, Inflow Depth > 0.75" for 2yr event
Inflow	=	147.84 cfs @ 12.74 hrs, Volume= 30.925 af
Outflow	=	25.57 cfs @ 12.18 hrs, Volume= 30.924 af, Atten= 83%, Lag= 0.0 min

Existing Prepared by {enter your company name here} HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLC

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 2.49 fps, Min. Travel Time= 0.4 min Avg. Velocity = 1.24 fps, Avg. Travel Time= 0.8 min

Peak Storage= 687 cf @ 12.18 hrs Average Depth at Peak Storage= 4.50' above invert (3.00' above fill) Bank-Full Depth= 4.50' above invert (3.00' above fill) Flow Area= 11.3 sf, Capacity= 24.84 cfs

54.0" Round Pipe w/ 18.0" inside fill n= 0.025 Corrugated metal Length= 61.0' Slope= 0.0016 '/' (101 Elevation Intervals) Inlet Invert= 542.00', Outlet Invert= 541.90'



Summary for Reach 7R: Rt 104 Westerly Culvert

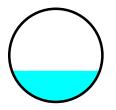
[52] Hint: Inlet/Outlet conditions not evaluated

Inflow Area =	=	86.790 ac, 1	17.76% Impe	ervious,	Inflow De	epth =	0.63"	for 2yr event	
Inflow =	=	29.18 cfs @	12.43 hrs,	Volume=		4.571 a	af		
Outflow =	=	29.16 cfs @	12.43 hrs,	Volume=		4.571 a	af, Atte	n= 0%, Lag= 0.	1 min
Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs									
Mary Malasia	Mary Valasity 4445 fee. Min. Travel Times, 04 min								

Max. Velocity= 14.15 fps, Min. Travel Time= 0.1 min Avg. Velocity = 4.93 fps, Avg. Travel Time= 0.4 min

Peak Storage= 247 cf @ 12.43 hrs Average Depth at Peak Storage= 1.00' Bank-Full Depth= 3.00' Flow Area= 7.1 sf, Capacity= 121.77 cfs

36.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 120.0' Slope= 0.0333 '/' Inlet Invert= 640.00', Outlet Invert= 636.00'



Summary for Reach 8R: Central Culvert into Pond

[52] Hint: Inlet/Outlet conditions not evaluated[55] Hint: Peak inflow is 771% of Manning's capacity[76] Warning: Detained 2.875 af (Pond w/culvert advised)

 Inflow Area =
 119.930 ac, 22.48% Impervious, Inflow Depth =
 0.78" for 2yr event

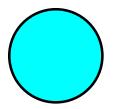
 Inflow =
 53.99 cfs @
 12.43 hrs, Volume=
 7.781 af

 Outflow =
 7.30 cfs @
 11.90 hrs, Volume=
 7.781 af, Atten= 86%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 5.72 fps, Min. Travel Time= 0.2 min Avg. Velocity = 3.01 fps, Avg. Travel Time= 0.3 min

Peak Storage= 84 cf @ 11.91 hrs Average Depth at Peak Storage= 1.33' Bank-Full Depth= 1.33' Flow Area= 1.4 sf, Capacity= 7.00 cfs

16.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 60.0' Slope= 0.0083 '/' Inlet Invert= 542.50', Outlet Invert= 542.00'



Summary for Reach 9R: Westerly culvert into Pond

[52] Hint: Inlet/Outlet conditions not evaluated

 Inflow Area =
 32.040 ac, 33.21% Impervious, Inflow Depth =
 1.16" for 2yr event

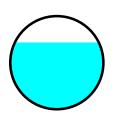
 Inflow =
 24.40 cfs @
 12.42 hrs, Volume=
 3.104 af

 Outflow =
 24.40 cfs @
 12.42 hrs, Volume=
 3.104 af, Atten= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 18.03 fps, Min. Travel Time= 0.1 min Avg. Velocity = 8.01 fps, Avg. Travel Time= 0.1 min

Peak Storage= 74 cf @ 12.42 hrs Average Depth at Peak Storage= 1.07' Bank-Full Depth= 1.50' Flow Area= 1.8 sf, Capacity= 28.33 cfs

18.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 55.0' Slope= 0.0727 '/' Inlet Invert= 544.00', Outlet Invert= 540.00'



#1

778.00'

Summary for Reach 10R: (new Reach)

Inflow Area = 130.360 ac, 7.73% Impervious, Inflow Depth = 0.65" for 2yr event Inflow = 38.50 cfs @ 12.75 hrs, Volume= 7.028 af Outflow = 38.49 cfs @ 12.76 hrs, Volume= 7.028 af, Atten= 0%, Lag= 0.7 min								
Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 4.22 fps, Min. Travel Time= 0.5 min Avg. Velocity = 2.06 fps, Avg. Travel Time= 0.9 min								
Peak Storage= 1,057 cf @ 12.75 hrs Average Depth at Peak Storage= 1.31' Bank-Full Depth= 2.00' Flow Area= 18.0 sf, Capacity= 96.39 cfs								
3.00' x 2.00' deep channel, n= 0.040 Mountain streams Side Slope Z-value= 3.0 '/' Top Width= 15.00' Length= 116.0' Slope= 0.0172 '/' Inlet Invert= 546.00', Outlet Invert= 544.00'								
‡								
Summary for Pond 1P: Meredith Reservoir								
Inflow Area = 123.050 ac, 3.86% Impervious, Inflow Depth = 0.56" for 2yr event Inflow = 39.23 cfs @ 12.47 hrs, Volume= 5.787 af Outflow = 8.21 cfs @ 13.99 hrs, Volume= 5.570 af, Atten= 79%, Lag= 91.4 min Primary = 8.21 cfs @ 13.99 hrs, Volume= 5.570 af								
Routing by Stor-Ind method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 778.64' @ 13.99 hrs Surf.Area= 3.438 ac Storage= 2.199 af								
Plug-Flow detention time= 264.4 min calculated for 5.570 af (96% of inflow) Center-of-Mass det. time= 244.6 min (1,154.3 - 909.7)								
Volume Invert Avail.Storage Storage Description								

14.178 af **Custom Stage Data (Prismatic)** Listed below (Recalc)

Existing

Prepared by {enter your company name here} HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLC

				Cum.Store (acre-feet)	
00	3.387		0.000	0.000	
00	3.702		14.178	14.178	
Routing		Invert			
Primary		778.00'			
					edge neadwall, Ke= 0.500 3.00' / 777.40' S= 0.0200 '/' Cc= 0.900
					E, smooth interior, Flow Area= 4.91 sf
	et) D0 D0 Routing	(acres) 00 3.387 00 3.702 Routing	et) (acres) (ac 00 3.387 00 3.702 Routing Invert	et) (acres) (acre-feet) 00 3.387 0.000 00 3.702 14.178 Routing Invert Outlet Primary 778.00' 30.0" L= 30.0 Inlet / 0	et) (acres) (acre-feet) (acre-feet) 00 3.387 0.000 0.000 00 3.702 14.178 14.178 Routing Invert Outlet Devices Primary 778.00' 30.0" Round Culvert X L= 30.0' CMP, square Inlet / Outlet Invert= 778

Primary OutFlow Max=8.21 cfs @ 13.99 hrs HW=778.64' (Free Discharge) ←1=Culvert (Inlet Controls 8.21 cfs @ 2.73 fps)

Summary for Pond 2P: Wilcom Pond

Inflow Area =	39.030 ac, 12.13% Impervious, Inflow I	Depth = 0.74" for 2yr event
Inflow =	18.28 cfs @ 12.42 hrs, Volume=	2.393 af
Outflow =	1.99 cfs @ 15.64 hrs, Volume=	0.854 af, Atten= 89%, Lag= 193.6 min
Primary =	1.99 cfs @ 15.64 hrs, Volume=	0.854 af
Secondary =	0.00 cfs @ 2.00 hrs, Volume=	0.000 af

Routing by Stor-Ind method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs / 2 Peak Elev= 715.93' @ 15.64 hrs Surf.Area= 0.348 ac Storage= 1.603 af

Plug-Flow detention time= 387.3 min calculated for 0.854 af (36% of inflow) Center-of-Mass det. time= 239.8 min (1,131.7 - 891.9)

Volume	Invert A	vail.Stora	ge Stora	ge Description
#1	710.00'	1.626	af Custo	om Stage Data (Prismatic) Listed below (Recalc)
Elevatio	••••••••••		c.Store re-feet)	Cum.Store (acre-feet)
710.0	0 0.192		0.000	0.000
716.0	0 0.350		1.626	1.626
Device	Routing	Invert	Outlet De	vices
#1	Primary	715.75'	1.0" x 1.0	" Horiz. Orifice/Grate X 12.00 columns
			Limited to	s C= 0.600 in 24.0" x 24.0" Grate (25% open area) o weir flow at low heads
#2	Secondary	715.95'	Head (fee 2.50 3.00 Coef. (En	g x 5.0' breadth Broad-Crested Rectangular Weir et) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 0 3.50 4.00 4.50 5.00 5.50 iglish) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 6 2.68 2.70 2.74 2.79 2.88

Primary OutFlow Max=2.04 cfs @ 15.64 hrs HW=715.93' (Free Discharge) ←1=Orifice/Grate (Weir Controls 2.04 cfs @ 1.40 fps)

Secondary OutFlow Max=0.00 cfs @ 2.00 hrs HW=710.00' (Free Discharge)

Summary for Pond 3P: Monkey Pond

[61] Hint: Exceeded Reach 6R outlet invert by 1.32' @ 13.12 hrs
[62] Hint: Exceeded Reach 8R OUTLET depth by 0.45' @ 24.93 hrs
[62] Hint: Exceeded Reach 9R OUTLET depth by 2.74' @ 13.31 hrs

Inflow Area =	790.260 ac, 14.04% Impervious, Inflow	<pre>/ Depth > 0.75" for 2yr event</pre>
Inflow =	90.96 cfs @ 12.60 hrs, Volume=	49.695 af
Outflow =	68.42 cfs @ 13.12 hrs, Volume=	48.557 af, Atten= 25%, Lag= 31.1 min
Primary =	68.42 cfs @ 13.12 hrs, Volume=	48.557 af

Routing by Stor-Ind method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs / 2 Peak Elev= 543.22' @ 13.12 hrs Surf.Area= 1.758 ac Storage= 4.388 af Flood Elev= 545.00' Surf.Area= 2.000 ac Storage= 7.766 af

Plug-Flow detention time= 68.6 min calculated for 48.547 af (98% of inflow) Center-of-Mass det. time= 51.9 min (1,154.3 - 1,102.4)

Volume	Inve	ert Av	ail.Storag	ge Sto	torage Description	
#1	540.0	00'	7.766	af Cu	custom Stage Data (Prismatic) Listed below (Recalc)	
Elevatio	on Su	rf.Area		Store.		
(fee	et) ((acres)	(acr	e-feet)) (acre-feet)	
540.0	0	0.867		0.000) 0.000	
542.0	0	1.517		2.384	2.384	
544.0	0	1.910		3.427	7 5.811	
545.0	0	2.000		1.955	5 7.766	
Device	Routing		Invert	Outlet	t Devices	
#1	Primary	ļ	541.00'	48.0" I	Round RCP_Round 48"	
				L= 40.0	0.0' RCP, groove end projecting, Ke= 0.200	
				Inlet / C	Outlet Invert= 541.00' / 540.20' S= 0.0200 '/' Cc= 0.900	
					011 Concrete pipe, straight & clean, Flow Area= 12.57 sf	
#2	Primary		541.00'	48.0" I	Round Steel Culvert	
				L= 47.0	'.0' CMP, projecting, no headwall, Ke= 0.900	
				Inlet / C	'Outlet Invert= 541.00' / 540.00' S= 0.0213 '/' Cc= 0.900	
				n= 0.01	012 Steel, smooth, Flow Area= 12.57 sf	
	0		0.40.56	a 40.40	$12 \text{ bra} = HW/-E42(22)^2$ (Erec Discharge)	

Primary OutFlow Max=68.42 cfs @ 13.12 hrs HW=543.22' (Free Discharge) -1=RCP_Round 48" (Barrel Controls 39.65 cfs @ 7.99 fps) -2=Steel Culvert (Inlet Controls 28.77 cfs @ 4.01 fps)

Summary for Link 2L: Lake Waukewan

 Inflow Area =
 790.260 ac, 14.04% Impervious, Inflow Depth > 0.74" for 2yr event

 Inflow =
 68.42 cfs @ 13.12 hrs, Volume=
 48.557 af

 Primary =
 68.42 cfs @ 13.12 hrs, Volume=
 48.557 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs

Existing
Prepared by {enter your company name here}
HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLC

Time span=2.00-48.00 hrs, dt=0.01 hrs, 4601 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Subcat 1Runoff Area=123.050 ac3.86% ImperviousRunoff Depth=1.28"Flow Length=2,832'Slope=0.1200 '/'Tc=28.2 minCN=70Runoff=101.84 cfs13.158 af
Subcatchment 2S: Subcat 2Runoff Area=203.010 ac5.87% ImperviousRunoff Depth=1.48"Flow Length=4,382'Slope=0.0950 '/'Tc=41.3 minCN=73Runoff=165.09 cfs24.994 af
Subcatchment 3S: Subcat 3Runoff Area=39.030 ac12.13% ImperviousRunoff Depth=1.55"Flow Length=2,511'Slope=0.0910 '/'Tc=26.3 minCN=74Runoff=41.30 cfs5.026 af
Subcatchment 4S: Subcat 4Runoff Area=47.760 ac22.35% ImperviousRunoff Depth=1.83"Flow Length=3,272'Slope=0.0980 '/'Tc=27.9 minCN=78Runoff=59.40 cfs7.289 af
Subcatchment 5S: Subcat 5Runoff Area=140.300 ac26.00% ImperviousRunoff Depth=1.91"Flow Length=3,798'Slope=0.0810 '/'Tc=33.5 minCN=79Runoff=167.98 cfs22.294 af
Subcatchment 6S: Subcat 6Runoff Area=130.360 ac7.73% ImperviousRunoff Depth=1.41"Flow Length=4,593'Slope=0.0810 '/'Tc=47.8 minCN=72Runoff=93.01 cfs15.330 af
Subcatchment 7S: Subcat 7Runoff Area=32.040 ac33.21% ImperviousRunoff Depth=2.14"Flow Length=3,154'Slope=0.0630 '/'Tc=29.8 minCN=82Runoff=45.73 cfs5.723 af
Subcatchment 8S: Subcat 8Runoff Area=33.140 ac34.86% ImperviousRunoff Depth=2.14"Flow Length=2,745'Slope=0.0470 '/'Tc=30.8 minCN=82Runoff=46.59 cfs5.919 af
Subcatchment 9S: Subcat 9Runoff Area=29.180 ac22.05% ImperviousRunoff Depth=1.76"Flow Length=2,576'Slope=0.0700 '/'Tc=28.0 minCN=77Runoff=34.68 cfs4.273 af
Subcatchment 10S: Subcat 10Runoff Area=12.390 ac29.64% ImperviousRunoff Depth=1.69"Flow Length=961'Slope=0.0830 '/'Tc=12.1 minCN=76Runoff=19.75 cfs1.740 af
Reach 1R: Reservoir Brook Avg. Flow Depth=0.97' Max Vel=6.09 fps Inflow=32.68 cfs 12.928 af n=0.040 L=2,356.0' S=0.0398 '/' Capacity=202.98 cfs Outflow=32.52 cfs 12.918 af
Reach 2R: Res Rd Culvert Avg. Flow Depth=2.83' Max Vel=18.37 fps Inflow=174.72 cfs 37.913 af 48.0" Round Pipe n=0.011 L=62.0' S=0.0147 '/' Capacity=205.67 cfs Outflow=174.71 cfs 37.913 af
Reach 3R: Reservoir Brook Avg. Flow Depth=3.17' Max Vel=8.90 fps Inflow=174.71 cfs 37.913 af n=0.040 L=1,983.0' S=0.0300 '/' Capacity=156.15 cfs Outflow=173.35 cfs 37.906 af
Reach 4R: Rt 104 Easterly Culvert Avg. Flow Depth=2.02' Max Vel=37.17 fps Inflow=311.61 cfs 60.200 af 72.0" Round Pipe n=0.011 L=124.0' S=0.0645 '/' Capacity=1,271.30 cfs Outflow=311.60 cfs 60.200 af
Reach 5R: Reservoir Brook Avg. Flow Depth=3.84' Max Vel=10.52 fps Inflow=311.60 cfs 60.200 af n=0.040 L=2,167.0' S=0.0332 '/' Capacity=201.12 cfs Outflow=308.76 cfs 60.192 af
Reach 6R: Easterly Culvert into PondAvg. Flow Depth=3.00'Max Vel=2.49 fpsInflow=331.24 cfs64.465 af54.0" Round Pipe w/ 18.0" inside fill n=0.025L=61.0'S=0.0016 '/'Capacity=24.84 cfsOutflow=26.42 cfs64.465 af

ExistingType III 24-hr10yr Rainfall=3.93"Prepared by {enter your company name here}Printed 12/23/2020HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLCPage 22
Reach 7R: Rt 104 Westerly Culvert Avg. Flow Depth=1.95' Max Vel=18.95 fps Inflow=93.02 cfs 11.182 af 36.0" Round Pipe n=0.013 L=120.0' S=0.0333 '/' Capacity=121.77 cfs Outflow=91.02 cfs 11.182 af
Reach 8R: Central Culvert into Pond Avg. Flow Depth=1.33' Max Vel=5.72 fps Inflow=133.77 cfs 17.101 af 16.0" Round Pipe n=0.013 L=60.0' S=0.0083 '/ Capacity=7.00 cfs Outflow=7.51 cfs 17.101 af
Reach 9R: Westerly culvert into Pond Avg. Flow Depth=1.50' Max Vel=18.27 fps Inflow=45.73 cfs 5.723 af 18.0" Round Pipe n=0.013 L=55.0' S=0.0727 '/' Capacity=28.33 cfs Outflow=29.93 cfs 5.723 af
Reach 10R: (new Reach) Avg. Flow Depth=1.97' Max Vel=5.31 fps Inflow=93.01 cfs 15.330 af n=0.040 L=116.0' S=0.0172 '/' Capacity=96.39 cfs Outflow=92.95 cfs 15.330 af
Pond 1P: Meredith Reservoir Peak Elev=779.37' Storage=4.700 af Inflow=101.84 cfs 13.158 af 30.0" Round Culvert x 3.00 n=0.013 L=30.0' S=0.0200 '/' Outflow=32.68 cfs 12.928 af
Pond 2P: Wilcom PondPeak Elev=717.06' Storage=1.626 af Inflow=41.30 cfs 5.026 af Primary=5.52 cfs 2.191 af Secondary=31.39 cfs 1.702 af Outflow=36.91 cfs 3.893 af
Pond 3P: Monkey PondPeak Elev=544.38' Storage=6.551 af Inflow=157.51 cfs 104.359 af Outflow=131.46 cfs 103.137 af
Link 2L: Lake Waukewan Inflow=131.46 cfs 103.137 af Primary=131.46 cfs 103.137 af
Total Runoff Area = 790.260 ac Runoff Volume = 105.746 af Average Runoff Depth = 1.61"

Total Runoff Area = 790.260 ac Runoff Volume = 105.746 af Average Runoff Depth = 1.61" 85.96% Pervious = 679.318 ac 14.04% Impervious = 110.942 ac

Summary for Subcatchment 1S: Subcat 1

Runoff = 101.84 cfs @ 12.44 hrs, Volume= 13.158 af, Depth= 1.28"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10yr Rainfall=3.93"

Area	(ac)	CN	Desc	ription			
14.	820	55	Woo	ds, Good,	HSG B		
74.	760	70	Woo	ds, Good,	HSG C		
16.	140	73	Woo	ds, Fair, H	ISG C		
5.	100	61	>75%	6 Grass co	over, Good,	HSG B	
2.	070	74	>75%	6 Grass co	over, Good,	HSG C	
1.	290	79	50-75	5% Grass	cover, Fair	, HSG C	
3.	310	92	Urba	n commei	cial, 85% iı	mp, HSG B	
1.	700	94				mp, HSG C	
0.	580	94	Urba	n commei	cial, 85% iı	mp, HSG C	
3.	280	98	Wate	er Surface	<u>, 0% imp, F</u>	ISG A	
123.	050	70	Weig	hted Aver	age		
118.	298		96.14	1% Pervio	us Area		
4.	751		3.86%	% Impervi	ous Area		
Tc	Length		Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
28.2	2,832	2 0.	1200	1.68		Lag/CN Method,	

Summary for Subcatchment 2S: Subcat 2

Runoff = 165.09 cfs @ 12.62 hrs, Volume= 24.994 af, Depth= 1.48"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10yr Rainfall=3.93"

Area (ac)	CN	Description
118.170	70	Woods, Good, HSG C
37.880	73	Woods, Fair, HSG C
23.520	77	Woods, Good, HSG D
7.540	74	>75% Grass cover, Good, HSG C
0.900	79	50-75% Grass cover, Fair, HSG C
11.110	94	Urban commercial, 85% imp, HSG C
2.920	95	Urban commercial, 85% imp, HSG D
0.970	98	Water Surface, 0% imp, HSG A
203.010	73	Weighted Average
191.085		94.13% Pervious Area
11.925		5.87% Impervious Area

	ed by {en .D® 10.00-		.,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	Printed 12/23/2020 Page 24			
Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
41.3	4,382	0.0950	1.77		Lag/CN Method,		

Summary for Subcatchment 3S: Subcat 3

Type III 24-hr 10yr Rainfall=3.93"

Runoff = 41.30 cfs @ 12.39 hrs, Volume= 5.026 af, Depth= 1.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10yr Rainfall=3.93"

 Area	(ac)	CN	Desc	ription		
30.	940	70	Woo	ds, Good,	HSG C	
2.	520	74	>75%	6 Grass co	over, Good,	I, HSG C
 5.	570	94	Urba	n commer	cial, 85% ir	imp, HSG C
39.	030	74	Weig	hted Aver	age	
34.	295		87.87	7% Pervio	us Area	
4.	734		12.13	3% Imperv	vious Area	
_					a 1.	
Tc	Lengt		Slope	Velocity	Capacity	Description
 (min)	(feet	:)	(ft/ft)	(ft/sec)	(cfs)	
26.3	2,51	1 0.	.0910	1.59		Lag/CN Method,
						-

Summary for Subcatchment 4S: Subcat 4

Runoff	=	59.40 cfs @	12.40 hrs,	Volume=	7.289 af,	Depth= 1.83"
--------	---	-------------	------------	---------	-----------	--------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10yr Rainfall=3.93"

Area (a	c) C	N De	Description				
20.23	30 7	70 Wo	ods, Good,	HSG C			
7.89	90 7	73 Wo	ods, Fair, F	ISG C			
7.08	30 7	77 Wo	ods, Good,	HSG D			
7.40	00 9	94 Url	ban comme	rcial, 85% i	mp, HSG C		
2.34	40 9	94 Url	ban comme	rcial, 85% i	mp, HSG C		
2.82	20 9	95 Url	ban comme	rcial, 85% i	mp, HSG D		
47.76	60 7	78 We	eighted Ave	rage			
37.08	34	77.	65% Pervio	us Area			
10.67	76	22.	35% Imperv	vious Area			
Tc L	_ength	Slope	e Velocity	Capacity	Description		
(min)	(feet)	(ft/ft) (ft/sec)	(cfs)			
27.9	3,272	0.0980) 1.96		Lag/CN Method,		

Existing

Summary for Subcatchment 5S: Subcat 5

Runoff = 167.98 cfs @ 12.47 hrs, Volume= 22.294 af, Depth= 1.91"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10yr Rainfall=3.93"

Area (ac)	CN	Desc	cription					
25.990	70) Woo	Noods, Good, HSG C					
49.750	73	3 Woo	ds, Fair, H	ISG C				
8.780	77	7 Woo	ds, Good,	HSG D				
7.760	74	4 >75%	% Grass co	over, Good,	, HSG C			
0.550	79	9 50-7	5% Grass	cover, Fair	, HSG C			
4.560	80) >75%	% Grass co	over, Good,	, HSG D			
19.720	94	1 Urba	an commei	rcial, 85% i	mp, HSG C			
20.960	94	1 Urba	an commei	rcial, 85% i	mp, HSG C			
2.230	95	5 Urba	an commei	rcial, 85% i	mp, HSG D			
140.300	79) Weig	ghted Aver	age				
103.826		74.0	0% Pervio	us Area				
36.473		26.0	0% Imperv	vious Area				
			-					
Tc Le	ngth	Slope	Velocity	Capacity	Description			
(min) (f	eet)	(ft/ft)	(ft/sec)	(cfs)				
33.5 3	,798	0.0810	1.89		Lag/CN Method,			

Summary for Subcatchment 6S: Subcat 6

Runoff = 93.01 cfs @ 12.69 hrs, Volume= 15.330 af, Depth= 1.41"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10yr Rainfall=3.93"

Area	(ac) (CN	Descr	iption				
10.	260	55	Wood	Woods, Good, HSG B				
50.	460	70	Wood	s, Good,	HSG C			
49.	240	73	Wood	s, Fair, H	ISG C			
5.	250	74	>75%	Grass co	over, Good,	HSG C		
3.	290	79	50-759	% Grass	cover, Fair	, HSG C		
1.	970	92	Urban	commei	cial, 85% ir	mp, HSG B		
2.	560	94	Urban	commei	cial, 85% ir	mp, HSG C		
7.	330	94	Urban	commei	cial, 85% ir	mp, HSG C		
130.	360	72	Weigh	nted Aver	age			
120.	279		92.279	% Pervio	us Area			
10.	081		7.73%	Impervi	ous Area			
				-				
Тс	Length	S	Slope `	ope Velocity Capacity Description				
(min)	(feet)		(ft/ft)	(ft/sec)	(cfs)			
47.8	4,593	0.0	0810	1.60		Lag/CN Method,		

Summary for Subcatchment 7S: Subcat 7

Runoff = 45.73 cfs @ 12.42 hrs, Volume= 5.723 af, Depth= 2.14"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10yr Rainfall=3.93"

Area (ac)	CN	Desc	ription			
0.170	55	Woo	ds, Good,	HSG B		
1.060	70	Woo	ds, Good,	HSG C		
12.680	73	Woo	ds, Fair, H	ISG C		
5.510	77	Woo	ds, Good,	HSG D		
0.100	74	>75%	6 Grass co	over, Good,	HSG C	
5.730	94	Urba	n commei	rcial, 85% ir	mp, HSG C	
6.790	95	Urba	n commei	rcial, 85% ir	mp, HSG D	
32.040	82	Weig	hted Aver	age		
21.398		66.79	9% Pervio	us Area		
10.642		33.21	1% Imperv	vious Area		
Tc Len	gth	Slope	Velocity	Capacity	Description	
(min) (fe	et)	(ft/ft)	(ft/sec)	(cfs)		
29.8 3,1	54 0	.0630	1.77		Lag/CN Method,	

Summary for Subcatchment 8S: Subcat 8

Runoff	=	46.59 cfs @	12.42 hrs, Volume=	5.919 af, Depth= 2.14"
--------	---	-------------	--------------------	------------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10yr Rainfall=3.93"

Area	(ac) (CN	Desc	ription			
0.	270	70	Woo	ds, Good,	HSG C		
19.	120	73	Woo	ds, Fair, H	ISG C		
0.	160	77	Woo	ds, Good,	HSG D		
1.	540	94	Urba	n commei	rcial, 85% ir	mp, HSG C	
10.	730	94	Urba	n commei	rcial, 85% ir	mp, HSG C	
1.3	320	95	Urba	n commei	rcial, 85% ir	mp, HSG D	
33.	140	82	Weig	hted Aver	age		
21.	588		65.14	4% Pervio	us Area		
11.	551		34.86	5% Imperv	vious Area		
Тс	Length		Slope	Velocity	Capacity	Description	
(min)	(feet)		(ft/ft)	(ft/sec)	(cfs)	Description	
30.8	. ,		. /	1.48	(013)	Lag/CN Method,	
30.8	2,745	0.0	0470	1.40		Lay CN Method,	

Summary for Subcatchment 9S: Subcat 9

Runoff = 34.68 cfs @ 12.41 hrs, Volume= 4.273 af, Depth= 1.76"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10yr Rainfall=3.93"

 Area (a	c) (N	Desc	ription		
1.58	30	55	Woo	ds, Good,	HSG B	
16.60	00	73	Woo	ds, Fair, H	ISG C	
1.81	10	77	Woo	ds, Good,	HSG D	
1.62	20	61	>75%	6 Grass co	over, Good,	d, HSG B
0.73	30	92	Urba	n commer	cial, 85% ii	imp, HSG B
6.34	10	94	Urba	n commer	cial, 85% ii	imp, HSG C
 0.50	00	95	Urba	n commer	cial, 85% ii	imp, HSG D
29.18	30	77	Weig	hted Aver	age	
22.74	15		77.95	5% Pervio	us Area	
6.43	35		22.05	5% Imperv	vious Area	
Tc L	ength	5	Slope	Velocity	Capacity	Description
 (min)	(feet)		(ft/ft)	(ft/sec)	(cfs)	
28.0	2,576	0.	0700	1.53		Lag/CN Method,

Summary for Subcatchment 10S: Subcat 10

Runoff	=	19.75 cfs @	12.17 hrs, Volume=	1.740 af, Depth= 1.69"
--------	---	-------------	--------------------	------------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 10yr Rainfall=3.93"

Area (a	ac) C	N Des	Description				
3.5	40 5	5 Wo	ods, Good,	HSG B			
3.7	30 7	'3 Wo	ods, Fair, ⊢	ISG C			
0.1	10 6	61 >75	% Grass co	over, Good,	, HSG B		
1.6	60 9	02 Urb	an commei	rcial, 85% iı	mp, HSG B		
2.6	60 9	94 Urb	an commei	rcial, 85% iı	mp, HSG C		
0.6	90 9	98 Wa	er Surface	, 0% imp, H	ISG A		
12.3	90 7	'6 We	ghted Aver	age			
8.7	18	70.3	6% Pervio	us Area			
3.6	72	29.6	64% Imperv	/ious Area			
Tc I	Length	Slope	Velocity	Capacity	Description		
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
12.1	961	0.0830	1.33		Lag/CN Method,		

Summary for Reach 1R: Reservoir Brook

 Inflow Area =
 123.050 ac,
 3.86% Impervious, Inflow Depth >
 1.26" for 10yr event

 Inflow =
 32.68 cfs @
 13.09 hrs, Volume=
 12.928 af

 Outflow =
 32.52 cfs @
 13.29 hrs, Volume=
 12.918 af, Atten= 0%, Lag= 11.7 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 6.09 fps, Min. Travel Time= 6.5 min Avg. Velocity = 2.17 fps, Avg. Travel Time= 18.1 min

Peak Storage= 12,589 cf @ 13.18 hrs Average Depth at Peak Storage= 0.97' Bank-Full Depth= 3.00' Flow Area= 19.5 sf, Capacity= 202.98 cfs

5.00' x 3.00' deep channel, n= 0.040 Mountain streams Side Slope Z-value= 0.5 '/' Top Width= 8.00' Length= 2,356.0' Slope= 0.0398 '/' Inlet Invert= 776.00', Outlet Invert= 682.31'



Summary for Reach 2R: Res Rd Culvert

[52] Hint: Inlet/Outlet conditions not evaluated[62] Hint: Exceeded Reach 1R OUTLET depth by 2.24' @ 12.53 hrs

 Inflow Area =
 326.060 ac,
 5.11% Impervious, Inflow Depth >
 1.40" for 10yr event

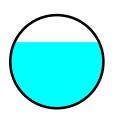
 Inflow =
 174.72 cfs @
 12.66 hrs, Volume=
 37.913 af

 Outflow =
 174.71 cfs @
 12.66 hrs, Volume=
 37.913 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs / 2 Max. Velocity= 18.37 fps, Min. Travel Time= 0.1 min Avg. Velocity = 6.04 fps, Avg. Travel Time= 0.2 min

Peak Storage= 590 cf @ 12.66 hrs Average Depth at Peak Storage= 2.83' Bank-Full Depth= 4.00' Flow Area= 12.6 sf, Capacity= 205.67 cfs

48.0" Round Pipe n= 0.011 Concrete pipe, straight & clean Length= 62.0' Slope= 0.0147 '/' Inlet Invert= 682.31', Outlet Invert= 681.40'



Summary for Reach 3R: Reservoir Brook

[91] Warning: Storage range exceeded by 0.17'
[55] Hint: Peak inflow is 112% of Manning's capacity
[62] Hint: Exceeded Reach 2R OUTLET depth by 0.51' @ 13.12 hrs

 Inflow Area =
 326.060 ac,
 5.11% Impervious, Inflow Depth >
 1.40" for 10yr event

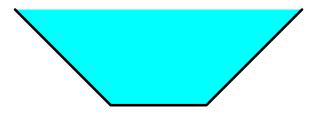
 Inflow =
 174.71 cfs @
 12.66 hrs, Volume=
 37.913 af

 Outflow =
 173.35 cfs @
 12.77 hrs, Volume=
 37.906 af, Atten= 1%, Lag= 6.5 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 8.90 fps, Min. Travel Time= 3.7 min Avg. Velocity = 2.77 fps, Avg. Travel Time= 11.9 min

Peak Storage= 38,639 cf @ 12.71 hrs Average Depth at Peak Storage= 3.17' Bank-Full Depth= 3.00' Flow Area= 18.0 sf, Capacity= 156.15 cfs

3.00' x 3.00' deep channel, n= 0.040 Mountain streams Side Slope Z-value= 1.0 '/' Top Width= 9.00' Length= 1,983.0' Slope= 0.0300 '/' Inlet Invert= 681.40', Outlet Invert= 622.00'



Summary for Reach 4R: Rt 104 Easterly Culvert

[52] Hint: Inlet/Outlet conditions not evaluated[62] Hint: Exceeded Reach 3R OUTLET depth by 0.23' @ 10.48 hrs

 Inflow Area =
 466.360 ac, 11.40% Impervious, Inflow Depth > 1.55" for 10yr event

 Inflow =
 311.61 cfs @ 12.61 hrs, Volume=
 60.200 af

 Outflow =
 311.60 cfs @ 12.61 hrs, Volume=
 60.200 af, Atten= 0%, Lag= 0.1 min

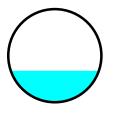
Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 37.17 fps, Min. Travel Time= 0.1 min Avg. Velocity = 10.67 fps, Avg. Travel Time= 0.2 min

Existing

Prepared by {enter your company name here} HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLC

Peak Storage= 1,039 cf @ 12.61 hrs Average Depth at Peak Storage= 2.02' Bank-Full Depth= 6.00' Flow Area= 28.3 sf, Capacity= 1,271.30 cfs

72.0" Round Pipe n= 0.011 Concrete pipe, straight & clean Length= 124.0' Slope= 0.0645 '/' Inlet Invert= 622.00', Outlet Invert= 614.00'



Summary for Reach 5R: Reservoir Brook

[91] Warning: Storage range exceeded by 0.84'

[55] Hint: Peak inflow is 155% of Manning's capacity

[62] Hint: Exceeded Reach 4R OUTLET depth by 1.83' @ 12.69 hrs

 Inflow Area =
 466.360 ac, 11.40% Impervious, Inflow Depth > 1.55" for 10yr event

 Inflow =
 311.60 cfs @ 12.61 hrs, Volume=
 60.200 af

 Outflow =
 308.76 cfs @ 12.72 hrs, Volume=
 60.192 af, Atten= 1%, Lag= 6.1 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 10.52 fps, Min. Travel Time= 3.4 min Avg. Velocity = 2.99 fps, Avg. Travel Time= 12.1 min

Peak Storage= 63,600 cf @ 12.66 hrs Average Depth at Peak Storage= 3.84' Bank-Full Depth= 3.00' Flow Area= 21.0 sf, Capacity= 201.12 cfs

4.00' x 3.00' deep channel, n= 0.040 Mountain streams Side Slope Z-value= 1.0 '/' Top Width= 10.00' Length= 2,167.0' Slope= 0.0332 '/' Inlet Invert= 614.00', Outlet Invert= 542.00'

Summary for Reach 6R: Easterly Culvert into Pond

[52] Hint: Inlet/Outlet conditions not evaluated

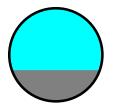
[55] Hint: Peak inflow is 1334% of Manning's capacity [76] Warning: Detained 38.273 af (Pond w/culvert advised)

[62] Hint: Exceeded Reach 5R OUTLET depth by 4.45' @ 42.53 hrs

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 2.49 fps, Min. Travel Time= 0.4 min Avg. Velocity = 1.89 fps, Avg. Travel Time= 0.5 min

Peak Storage= 687 cf @ 11.84 hrs Average Depth at Peak Storage= 4.50' above invert (3.00' above fill) Bank-Full Depth= 4.50' above invert (3.00' above fill) Flow Area= 11.3 sf, Capacity= 24.84 cfs

54.0" Round Pipe w/ 18.0" inside fill n= 0.025 Corrugated metal Length= 61.0' Slope= 0.0016 '/' (101 Elevation Intervals) Inlet Invert= 542.00', Outlet Invert= 541.90'



Summary for Reach 7R: Rt 104 Westerly Culvert

[52] Hint: Inlet/Outlet conditions not evaluated

 Inflow Area =
 86.790 ac, 17.76% Impervious, Inflow Depth =
 1.55" for 10yr event

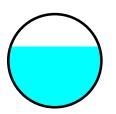
 Inflow =
 93.02 cfs @
 12.54 hrs, Volume=
 11.182 af

 Outflow =
 91.02 cfs @
 12.55 hrs, Volume=
 11.182 af, Atten= 2%, Lag= 0.5 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 18.95 fps, Min. Travel Time= 0.1 min Avg. Velocity = 5.95 fps, Avg. Travel Time= 0.3 min

Peak Storage= 585 cf @ 12.55 hrs Average Depth at Peak Storage= 1.95' Bank-Full Depth= 3.00' Flow Area= 7.1 sf, Capacity= 121.77 cfs

36.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 120.0' Slope= 0.0333 '/' Inlet Invert= 640.00', Outlet Invert= 636.00'



Summary for Reach 8R: Central Culvert into Pond

[52] Hint: Inlet/Outlet conditions not evaluated[55] Hint: Peak inflow is 1910% of Manning's capacity[76] Warning: Detained 10.286 af (Pond w/culvert advised)

 Inflow Area =
 119.930 ac, 22.48% Impervious, Inflow Depth =
 1.71" for 10yr event

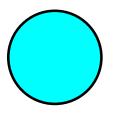
 Inflow =
 133.77 cfs @
 12.55 hrs, Volume=
 17.101 af

 Outflow =
 7.51 cfs @
 11.47 hrs, Volume=
 17.101 af, Atten= 94%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 5.72 fps, Min. Travel Time= 0.2 min Avg. Velocity = 4.85 fps, Avg. Travel Time= 0.2 min

Peak Storage= 84 cf @ 11.48 hrs Average Depth at Peak Storage= 1.33' Bank-Full Depth= 1.33' Flow Area= 1.4 sf, Capacity= 7.00 cfs

16.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 60.0' Slope= 0.0083 '/' Inlet Invert= 542.50', Outlet Invert= 542.00'



Summary for Reach 9R: Westerly culvert into Pond

[52] Hint: Inlet/Outlet conditions not evaluated

[55] Hint: Peak inflow is 161% of Manning's capacity

[76] Warning: Detained 0.489 af (Pond w/culvert advised)

[85] Warning: Oscillations may require smaller dt or Finer Routing (severity=1)

Inflow Area =	32.040 ac, 33.21% Impervious, Inflow	v Depth = 2.14" for 10yr event
Inflow =	45.73 cfs @ 12.42 hrs, Volume=	5.723 af
Outflow =	29.93 cfs @ 12.20 hrs, Volume=	5.723 af, Atten= 35%, Lag= 0.0 min

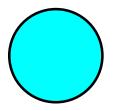
Existing Prepared by {enter your company name here} HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLC

Type III 24-hr 10yr Rainfall=3.93" Printed 12/23/2020 Page 33

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 18.27 fps, Min. Travel Time= 0.1 min Avg. Velocity = 9.02 fps, Avg. Travel Time= 0.1 min

Peak Storage= 97 cf @ 12.21 hrs Average Depth at Peak Storage= 1.50' Bank-Full Depth= 1.50' Flow Area= 1.8 sf, Capacity= 28.33 cfs

18.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 55.0' Slope= 0.0727 '/' Inlet Invert= 544.00', Outlet Invert= 540.00'



Summary for Reach 10R: (new Reach)

Inflow Area	a =	130.360 ac,	7.73% Impervious, Inflow	Depth = 1.41"	for 10yr event
Inflow	=	93.01 cfs @	12.69 hrs, Volume=	15.330 af	-
Outflow	=	92.95 cfs @	12.71 hrs, Volume=	15.330 af, Atte	en= 0%, Lag= 0.7 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 5.31 fps, Min. Travel Time= 0.4 min Avg. Velocity = 2.44 fps, Avg. Travel Time= 0.8 min

Peak Storage= 2,033 cf @ 12.70 hrs Average Depth at Peak Storage= 1.97' Bank-Full Depth= 2.00' Flow Area= 18.0 sf, Capacity= 96.39 cfs

3.00' x 2.00' deep channel, n= 0.040 Mountain streams Side Slope Z-value= 3.0 '/' Top Width= 15.00' Length= 116.0' Slope= 0.0172 '/' Inlet Invert= 546.00', Outlet Invert= 544.00'



Summary for Pond 1P: Meredith Reservoir

	123.050 ac, 101.84 cfs @ 32.68 cfs @ 32.68 cfs @	12.44 hrs, 13.09 hrs,	Volume= Volume=	Depth = 1.28" for 10yr event 13.158 af 12.928 af, Atten= 68%, Lag= 39.3 min 12.928 af		
Routing by Stor-Ir Peak Elev= 779.3						
Plug-Flow detenti Center-of-Mass d				af (98% of inflow)		
Volume Inv	ert Avail.St	orage Sto	rage Description	1		
#1 778.0	00' 14.'	178 af Cus	stom Stage Data	(Prismatic) Listed below (Recalc)		
	urf.Area (acres) 3.387 3.702	Inc.Store acre-feet) 0.000 14.178	Cum.Store (acre-feet) 0.000 14.178			
Device Routing		rt Outlet D				
#1 Primary		L= 30.0 Inlet / C n= 0.01	Outlet Invert= 778 3 Corrugated Pl	edge headwall, Ke= 0.500 3.00' / 777.40' S= 0.0200 '/' Cc= 0.900 E, smooth interior, Flow Area= 4.91 sf		
1=Culvert (Ba				7' (Free Discharge)		
		Summary	y for Pond 2P:	: Wilcom Pond		
[93] Warning: Sto	rage range ex	ceeded by	1.06'			
Inflow Area = 39.030 ac, 12.13% Impervious, Inflow Depth = 1.55" for 10yr event Inflow = 41.30 cfs @ 12.39 hrs, Volume= 5.026 af Outflow = 36.91 cfs @ 12.54 hrs, Volume= 3.893 af, Atten= 11%, Lag= 9.2 min Primary = 5.52 cfs @ 12.54 hrs, Volume= 2.191 af Secondary = 31.39 cfs @ 12.54 hrs, Volume= 1.702 af						
Routing by Stor-Ir Peak Elev= 717.0						
Plug-Flow detention time= 152.1 min calculated for 3.893 af (77% of inflow) Center-of-Mass det. time= 65.4 min (934.2 - 868.8)						

Volume	Invert	Avail.Storage	Storage Description
#1	710.00'	1.626 af	Custom Stage Data (Prismatic) Listed below (Recalc)

Existing

Prepared by {enter your company name here} HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLC

Elevatio (fee			c.Store re-feet)	Cum.Store (acre-feet)			
710.0			0.000	0.000			
Device	Routing	Invert	Outlet Dev	VICES			
#1	Primary	715.75'	1.0" x 1.0'	" Horiz. Orific	e/Grate X 12.00 columns		
	-		X 12 rows	C= 0.600 in 2	24.0" x 24.0" Grate (25% open area)		
			Limited to	weir flow at le	ow heads		
#2	Secondary	715.95'	10.0' long	x 5.0' bread	th Broad-Crested Rectangular Weir		
	-		Head (fee	t) 0.20 0.40	0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00		
			2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65				
			2.67 2.66	2.68 2.70 2	.74 2.79 2.88		
D '							

Primary OutFlow Max=5.52 cfs @ 12.54 hrs HW=717.06' (Free Discharge) ←1=Orifice/Grate (Orifice Controls 5.52 cfs @ 5.52 fps)

Secondary OutFlow Max=31.39 cfs @ 12.54 hrs HW=717.06' (Free Discharge) -2=Broad-Crested Rectangular Weir (Weir Controls 31.39 cfs @ 2.82 fps)

Summary for Pond 3P: Monkey Pond

[61] Hint: Exceeded Reach 6R outlet invert by 2.48' @ 13.04 hrs

[63] Warning: Exceeded Reach 8R INLET depth by 0.55' @ 13.04 hrs

[62] Hint: Exceeded Reach 9R OUTLET depth by 3.70' @ 13.28 hrs

[61] Hint: Exceeded Reach 10R outlet invert by 0.38' @ 13.04 hrs

Inflow Area =	790.260 ac, 14.04% Impervious, Inflo	ow Depth > 1.58" for 10yr event
Inflow =	157.51 cfs @ 12.70 hrs, Volume=	104.359 af
Outflow =	131.46 cfs @ 13.04 hrs, Volume=	103.137 af, Atten= 17%, Lag= 20.5 min
Primary =	131.46 cfs @ 13.04 hrs, Volume=	103.137 af

Routing by Stor-Ind method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs / 2 Peak Elev= 544.38' @ 13.04 hrs Surf.Area= 1.945 ac Storage= 6.551 af Flood Elev= 545.00' Surf.Area= 2.000 ac Storage= 7.766 af

Plug-Flow detention time= 61.4 min calculated for 103.137 af (99% of inflow) Center-of-Mass det. time= 48.3 min (1,486.1 - 1,437.8)

Volume	Invert	Avail.Storage	Stora	ge Description	
#1	540.00'	7.766 af	Custo	om Stage Data	(Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Are (acres			Cum.Store (acre-feet)	
540.00	0.86		000	0.000	
542.00	1.51		384	2.384	
544.00 545.00	1.91 2.00		427 955	5.811 7.766	

Existing

Type III 24-hr 10yr Rainfall=3.93" Printed 12/23/2020 Page 36

Prepared by {enter your company name here} HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLC

Device	Routing	Invert	Outlet Devices
#1	Primary	541.00'	48.0" Round RCP_Round 48"
			L= 40.0' RCP, groove end projecting, Ke= 0.200
			Inlet / Outlet Invert= 541.00' / 540.20' S= 0.0200 '/' Cc= 0.900
			n= 0.011 Concrete pipe, straight & clean, Flow Area= 12.57 sf
#2	Primary	541.00'	48.0" Round Steel Culvert
			L= 47.0' CMP, projecting, no headwall, Ke= 0.900
			Inlet / Outlet Invert= 541.00' / 540.00' S= 0.0213 '/' Cc= 0.900
			n= 0.012 Steel, smooth, Flow Area= 12.57 sf

Primary OutFlow Max=131.46 cfs @ 13.04 hrs HW=544.38' (Free Discharge) 1=RCP_Round 48" (Barrel Controls 75.39 cfs @ 8.97 fps) 2=Steel Culvert (Inlet Controls 56.07 cfs @ 4.94 fps)

Summary for Link 2L: Lake Waukewan

Inflow Area =	790.260 ac, 14.04% Impervious, Inflow	Depth > 1.57" for 10yr event
Inflow =	131.46 cfs @ 13.04 hrs, Volume=	103.137 af
Primary =	131.46 cfs @ 13.04 hrs, Volume=	103.137 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs

Existing
Prepared by {enter your company name here}
HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLC

Time span=2.00-48.00 hrs, dt=0.01 hrs, 4601 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Subcat 1Runoff Area=123.050 ac3.86% ImperviousRunoff Depth=1.93"Flow Length=2,832'Slope=0.1200 '/'Tc=28.2 minCN=70Runoff=157.57 cfs19.748 af
Subcatchment 2S: Subcat 2Runoff Area=203.010 ac5.87% ImperviousRunoff Depth=2.16"Flow Length=4,382'Slope=0.0950 '/'Tc=41.3 minCN=73Runoff=246.12 cfs36.601 af
Subcatchment 3S: Subcat 3Runoff Area=39.030 ac12.13% ImperviousRunoff Depth=2.25"Flow Length=2,511'Slope=0.0910 '/'Tc=26.3 minCN=74Runoff=61.11 cfs7.303 af
Subcatchment 4S: Subcat 4Runoff Area=47.760 ac22.35% ImperviousRunoff Depth=2.58"Flow Length=3,272'Slope=0.0980 '/'Tc=27.9 minCN=78Runoff=84.44 cfs10.288 af
Subcatchment 5S: Subcat 5Runoff Area=140.300 ac26.00% ImperviousRunoff Depth=2.67"Flow Length=3,798'Slope=0.0810 '/'Tc=33.5 minCN=79Runoff=236.68 cfs31.251 af
Subcatchment 6S: Subcat 6Runoff Area=130.360 ac7.73% ImperviousRunoff Depth=2.08"Flow Length=4,593'Slope=0.0810 '/'Tc=47.8 minCN=72Runoff=140.56 cfs22.628 af
Subcatchment 7S: Subcat 7Runoff Area=32.040 ac33.21% ImperviousRunoff Depth=2.95"Flow Length=3,154'Slope=0.0630 '/'Tc=29.8 minCN=82Runoff=62.80 cfs7.864 af
Subcatchment 8S: Subcat 8Runoff Area=33.140 ac34.86% ImperviousRunoff Depth=2.95"Flow Length=2,745'Slope=0.0470 '/'Tc=30.8 minCN=82Runoff=64.02 cfs8.134 af
Subcatchment 9S: Subcat 9Runoff Area=29.180 ac 22.05% Impervious Runoff Depth=2.50"Flow Length=2,576'Slope=0.0700 '/' Tc=28.0 min CN=77 Runoff=49.70 cfs 6.074 af
Subcatchment 10S: Subcat 10Runoff Area=12.390 ac29.64% ImperviousRunoff Depth=2.41"Flow Length=961'Slope=0.0830 '/'Tc=12.1 minCN=76Runoff=28.63 cfs2.491 af
Reach 1R: Reservoir Brook Avg. Flow Depth=1.40' Max Vel=7.33 fps Inflow=58.93 cfs 19.511 af n=0.040 L=2,356.0' S=0.0398 '/' Capacity=202.98 cfs Outflow=58.65 cfs 19.502 af
Reach 2R: Res Rd Culvert Avg. Flow Depth=4.00' Max Vel=18.66 fps Inflow=277.03 cfs 56.103 af 48.0" Round Pipe n=0.011 L=62.0' S=0.0147 '/' Capacity=205.67 cfs Outflow=217.90 cfs 56.090 af
Reach 3R: Reservoir Brook Avg. Flow Depth=3.48' Max Vel=9.23 fps Inflow=217.90 cfs 56.090 af n=0.040 L=1,983.0' S=0.0300 '/' Capacity=156.15 cfs Outflow=205.67 cfs 56.083 af
Reach 4R: Rt 104 Easterly Culvert Avg. Flow Depth=2.42' Max Vel=40.73 fps Inflow=434.83 cfs 87.334 af 72.0" Round Pipe n=0.011 L=124.0' S=0.0645 '/' Capacity=1,271.30 cfs Outflow=434.80 cfs 87.334 af
Reach 5R: Reservoir Brook Avg. Flow Depth=4.77' Max Vel=11.09 fps Inflow=434.80 cfs 87.334 af n=0.040 L=2,167.0' S=0.0332 '/' Capacity=201.12 cfs Outflow=428.01 cfs 87.326 af
Reach 6R: Easterly Culvert into PondAvg. Flow Depth=3.00'Max Vel=2.49 fpsInflow=465.83 cfs93.401 af54.0" Round Pipe w/ 18.0" inside fill n=0.025L=61.0'S=0.0016 '/'Capacity=24.84 cfsOutflow=26.58 cfs76.992 af

Existing Prepared by {enter your company name here} HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLC	Type III 24-hr 25yr Rainfall=4.85" Printed 12/23/2020 Page 38
Reach 7R: Rt 104 Westerly CulvertAvg. Flow Depth=2.63' Max Ve36.0" Round Pipen=0.013L=120.0'S=0.0333 '/'Capacity=1	
Reach 8R: Central Culvert into PondAvg. Flow Depth=1.33'Max V16.0"Round Pipen=0.013L=60.0'S=0.0083 '/'Capacity	•
Reach 9R: Westerly culvert into PondAvg. Flow Depth=1.50'Max '18.0" Round Pipen=0.013L=55.0'S=0.0727 '/'Capacit	
Reach 10R: (new Reach) Avg. Flow Depth=2.41' Max V n=0.040 L=116.0' S=0.0172 '/' Capacity=	el=5.82 fps Inflow=140.56 cfs 22.628 af 96.39 cfs Outflow=140.46 cfs 22.628 af
Pond 1P: Meredith ReservoirPeak Elev=780.00' Storag30.0" Round Culvert x 3.00n=0.013L=30.0' S	ge=6.939 af Inflow=157.57 cfs 19.748 af =0.0200 '/' Outflow=58.93 cfs 19.511 af
Pond 2P: Wilcom PondPeak Elev=717.16' StorPrimary=5.71 cfs2.271 afSecondary=35.23 c	rage=1.626 af Inflow=61.11 cfs 7.303 af cfs 1.019 af Outflow=40.94 cfs 3.291 af
Pond 3P: Monkey PondPeak Elev=545.89'Storage	e=7.766 af Inflow=206.77 cfs 131.687 af Outflow=202.15 cfs 128.891 af
Link 2L: Lake Waukewan	Inflow=202.15 cfs 128.891 af Primary=202.15 cfs 128.891 af
Total Runoff Area = 790.260 ac Runoff Volume = 152.3	81 af Average Runoff Depth = 2.31"

Total Runoff Area = 790.260 ac Runoff Volume = 152.381 af Average Runoff Depth = 2.31" 85.96% Pervious = 679.318 ac 14.04% Impervious = 110.942 ac

Summary for Subcatchment 1S: Subcat 1

Runoff = 157.57 cfs @ 12.41 hrs, Volume= 19.748 af, Depth= 1.93"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25yr Rainfall=4.85"

Area (a	ac) C	N Des	cription			
14.8	20 5	55 Wo	ods, Good,	HSG B		
74.7	60 7	'0 Wo	ods, Good,	HSG C		
16.1	40 7	'3 Wo	ods, Fair, F	ISG C		
5.1	00 6	61 >75	% Grass co	over, Good	, HSG B	
2.0	70 7	'4 >75	% Grass co	over, Good	, HSG C	
1.2	90 7	' 9 50-7	75% Grass	cover, Fair	, HSG C	
3.3	510 S	02 Urb	an comme	rcial, 85% i	mp, HSG B	
1.7	00 9				mp, HSG C	
0.5	80 9	04 Urb	an comme	rcial, 85% i	mp, HSG C	
3.2	80 9	98 Wat	er Surface	<u>, 0% imp, F</u>	ISG A	
123.0	50 7	′0 Wei	ghted Avei	age		
118.2	98	96.1	4% Pervio	us Area		
4.7	51	3.86	% Impervi	ous Area		
Тс	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
28.2	2,832	0.1200	1.68		Lag/CN Method,	
					-	

Summary for Subcatchment 2S: Subcat 2

Runoff = 246.12 cfs @ 12.58 hrs, Volume= 36.601 af, Depth= 2.16"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25yr Rainfall=4.85"

Area (ac)	CN	Description
118.170	70	Woods, Good, HSG C
37.880	73	Woods, Fair, HSG C
23.520	77	Woods, Good, HSG D
7.540	74	>75% Grass cover, Good, HSG C
0.900	79	50-75% Grass cover, Fair, HSG C
11.110	94	Urban commercial, 85% imp, HSG C
2.920	95	Urban commercial, 85% imp, HSG D
0.970	98	Water Surface, 0% imp, HSG A
203.010	73	Weighted Average
191.085		94.13% Pervious Area
11.925		5.87% Impervious Area

Prepared by {ent	er your compar	Printed 12/23/2020		
HydroCAD® 10.00-	18 s/n 00761 © 2	Page 40		
Tc Length	Slope Veloci		Description	

Summary for Subcatchment 3S: Subcat 3

Lag/CN Method,

Type III 24-hr 25yr Rainfall=4.85"

61.11 cfs @ 12.37 hrs, Volume= 7.303 af, Depth= 2.25" Runoff =

1.77

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25yr Rainfall=4.85"

 Area	(ac)	CN	Desc	cription		
30.	940	70	Woo	ds, Good,	HSG C	
2.	520	74	>75%	6 Grass co	over, Good,	I, HSG C
 5.	570	94	Urba	in commei	<u>cial, 85% ir</u>	imp, HSG C
39.	030	74	Weig	ghted Aver	age	
34.295 87.87% Pervious Area				7% Pervio	us Area	
4.734			12.13	3% Imperv	vious Area	
т.	1				0	Description
Tc	Lengt		Slope	Velocity	Capacity	Description
 (min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	
26.3	2,51	1 0.	.0910	1.59		Lag/CN Method,

Summary for Subcatchment 4S: Subcat 4

Runoff	=	84.44 cfs @	12.38 hrs,	Volume=	10.288 af, Depth= 2.58"
--------	---	-------------	------------	---------	-------------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25yr Rainfall=4.85"

Area (ac)	CI	N Dese	cription			
20.230	7	0 Woo	ds, Good,	HSG C		
7.890	7	3 Woo	ds, Fair, ⊢	ISG C		
7.080	7	7 Woo	ds, Good,	HSG D		
7.400	9	4 Urba	an commei	rcial, 85% ir	mp, HSG C	
2.340	9	4 Urba	an commei	rcial, 85% ir	mp, HSG C	
2.820	9	5 Urba	an commei	rcial, 85% ir	mp, HSG D	
47.760	7	8 Weig	ghted Aver	age		
37.084		77.6	5% Pervio	us Area		
10.676		22.3	5% Imper\	vious Area		
Tc Le	ngth	Slope	Velocity	Capacity	Description	
(min) (1	eet)	(ft/ft)	(ft/sec)	(cfs)		
27.9 3	,272	0.0980	1.96		Lag/CN Method,	

Existing

41.3

4,382 0.0950

Summary for Subcatchment 5S: Subcat 5

Runoff = 236.68 cfs @ 12.47 hrs, Volume= 31.251 af, Depth= 2.67"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25yr Rainfall=4.85"

Area (ad	c) C	N Des	scription			
25.99	0 7					
49.75	50 7	'3 Wo	ods, Fair, F	ISG C		
8.78	80 7	7 Wo	ods, Good,	HSG D		
7.76	60 7	4 >75	% Grass co	over, Good	, HSG C	
0.55	50 7	9 50-	75% Grass	cover, Fair	, HSG C	
4.56	60 B	0 >75	% Grass co	over, Good	, HSG D	
19.72	20 9	4 Urb	an comme	rcial, 85% i	mp, HSG C	
20.96	60 S	4 Urb	an comme	rcial, 85% i	mp, HSG C	
2.23	80 S	5 Urb	an comme	rcial, 85% i	mp, HSG D	
140.30	0 7	'9 We	ighted Aver	age		
103.82	26	74.0	00% Pervio	us Area		
36.47	'3	26.0	00% Imperv	ious Area		
Tc L	.ength	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
33.5	3,798	0.0810	1.89		Lag/CN Method,	

Summary for Subcatchment 6S: Subcat 6

Runoff = 140.56 cfs @ 12.69 hrs, Volume= 22.628 af, Depth= 2.08"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25yr Rainfall=4.85"

Area (ac) C	N I	Description			
10.2	260	55 \	Woods, Good,	HSG B		
50.4	460	70 \	Woods, Good,	HSG C		
49.2	240	73 \	Woods, Fair, H	ISG C		
5.2	250	74 :	>75% Grass c	over, Good,	, HSG C	
3.2	290	79 !	50-75% Grass	cover, Fair	, HSG C	
1.9	970 9	92 I	Urban comme	rcial, 85% i	mp, HSG B	
2.5	560 9	94 I	Urban comme	rcial, 85% i	mp, HSG C	
7.3	330 9	94 I	Urban comme	rcial, 85% i	mp, HSG C	
130.3	360	72 \	Weighted Ave	rage		
120.2	279	ę	92.27% Pervic	ous Area		
10.0	081	-	7.73% Impervi	ious Area		
Тс	Length	Slo	ope Velocity	Capacity	Description	
(min)	(feet)	(f	t/ft) (ft/sec)	(cfs)		
47.8	4,593	0.08	810 1.60		Lag/CN Method,	

Summary for Subcatchment 7S: Subcat 7

Runoff = 62.80 cfs @ 12.42 hrs, Volume= 7.864 af, Depth= 2.95"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25yr Rainfall=4.85"

Area	(ac)	CN	Desc	ription			
0.	170	55	Woo	ds, Good,	HSG B		
1.	060	70	Woo	ds, Good,	HSG C		
12.	680	73	Woo	ds, Fair, H	ISG C		
5.	510	77	Woo	ds, Good,	HSG D		
0.	100	74	>75%	6 Grass co	over, Good,	HSG C	
5.	730	94	Urba	n commei	cial, 85% ir	mp, HSG C	
6.	790	95	Urba	n commei	cial, 85% ir	mp, HSG D	
32.	040	82	Weig	hted Aver	age		
21.	398		66.7	9% Pervio	us Area		
10.	642		33.2	1% Imperv	vious Area		
Tc	Leng	th	Slope	Velocity	Capacity	Description	
(min)	(fee	et)	(ft/ft)	(ft/sec)	(cfs)		
29.8	3,15	54 C	.0630	1.77		Lag/CN Method,	

Summary for Subcatchment 8S: Subcat 8

Runoff	=	64.02 cfs @	12.42 hrs, Volume=	8.134 af, Depth= 2.95"
--------	---	-------------	--------------------	------------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25yr Rainfall=4.85"

Area (a	ac) C	N De	scription			
0.2	270 7	70 W	oods, Good,	HSG C		
19.1	20 7	73 W	oods, Fair, F	ISG C		
0.1	60 7	77 W	oods, Good,	HSG D		
1.5	540 S	94 Ur	ban comme	rcial, 85% i	mp, HSG C	
10.7	'30 S	94 Ur	ban comme	rcial, 85% i	mp, HSG C	
1.3	320 9	95 Ur	ban comme	<u>rcial, 85% i</u>	mp, HSG D	
33.1	40 8	32 W	eighted Ave	rage		
21.5	88	65	.14% Pervic	us Area		
11.5	551	34	.86% Imper	vious Area		
Та	Longth	Clan		Consoit	Description	
	Length	Slop		Capacity	Description	
(min)	(feet)	(ft/f	, , ,	(cfs)		
30.8	2,745	0.047	0 1.48		Lag/CN Method,	

Summary for Subcatchment 9S: Subcat 9

Runoff = 49.70 cfs @ 12.41 hrs, Volume= 6.074 af, Depth= 2.50"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25yr Rainfall=4.85"

 Area (a	c) (CN	Desc	ription		
1.58	30	55	Woo	ds, Good,	HSG B	
16.60	00	73	Woo	ds, Fair, H	ISG C	
1.81	10	77	Woo	ds, Good,	HSG D	
1.62	20	61	>75%	6 Grass co	over, Good,	d, HSG B
0.73	30	92	Urba	n commer	cial, 85% ii	imp, HSG B
6.34	40	94	Urba	n commer	cial, 85% ii	imp, HSG C
 0.50	00	95	Urba	n commer	cial, 85% ii	imp, HSG D
29.18	30	77	Weig	hted Aver	age	
22.74	15		77.95	5% Pervio	us Area	
6.43	35		22.05	5% Imperv	vious Area	
Tc L	ength	5	Slope	Velocity	Capacity	Description
 (min)	(feet)		(ft/ft)	(ft/sec)	(cfs)	
28.0	2,576	0.	0700	1.53		Lag/CN Method,

Summary for Subcatchment 10S: Subcat 10

Runoff	=	28.63 cfs @	12.17 hrs, Volume=	2.491 af, Depth= 2.41"
--------	---	-------------	--------------------	------------------------

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 25yr Rainfall=4.85"

Area (a	ac) C	N D	escription			
3.5	540	55 W	oods, Good	HSG B		
3.7	' 30	73 W	oods, Fair, I	HSG C		
0.1	10	61 >	75% Grass c	over, Good	, HSG B	
1.6	60	92 U	rban comme	rcial, 85% i	mp, HSG B	
2.6	60	94 U	rban comme	rcial, 85% i	mp, HSG C	
0.6	6 9 0	98 V	ater Surface	e, 0% imp, H	ISG A	
12.3	390 [·]	76 W	eighted Ave	rage		
8.7	' 18	7	0.36% Pervic	ous Area		
3.6	672	2	9.64% Imper	vious Area		
Тс	Length	Slop	be Velocity	Capacity	Description	
(min)	(feet)	(ft/	ft) (ft/sec)	(cfs)		
12.1	961	0.08	30 1.33		Lag/CN Method,	

Summary for Reach 1R: Reservoir Brook

 Inflow Area =
 123.050 ac,
 3.86% Impervious, Inflow Depth >
 1.90" for 25yr event

 Inflow =
 58.93 cfs @
 12.97 hrs, Volume=
 19.511 af

 Outflow =
 58.65 cfs @
 13.13 hrs, Volume=
 19.502 af, Atten= 0%, Lag= 9.8 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 7.33 fps, Min. Travel Time= 5.4 min Avg. Velocity = 2.38 fps, Avg. Travel Time= 16.5 min

Peak Storage= 18,860 cf @ 13.04 hrs Average Depth at Peak Storage= 1.40' Bank-Full Depth= 3.00' Flow Area= 19.5 sf, Capacity= 202.98 cfs

5.00' x 3.00' deep channel, n= 0.040 Mountain streams Side Slope Z-value= 0.5 '/' Top Width= 8.00' Length= 2,356.0' Slope= 0.0398 '/' Inlet Invert= 776.00', Outlet Invert= 682.31'



Summary for Reach 2R: Res Rd Culvert

[52] Hint: Inlet/Outlet conditions not evaluated

[55] Hint: Peak inflow is 135% of Manning's capacity

[76] Warning: Detained 2.257 af (Pond w/culvert advised)

[62] Hint: Exceeded Reach 1R OUTLET depth by 3.33' @ 12.42 hrs

 Inflow Area =
 326.060 ac,
 5.11% Impervious, Inflow Depth > 2.06" for 25yr event

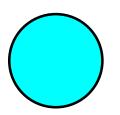
 Inflow =
 277.03 cfs @
 12.65 hrs, Volume=
 56.103 af

 Outflow =
 217.90 cfs @
 12.41 hrs, Volume=
 56.090 af, Atten= 21%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs / 2 Max. Velocity= 18.66 fps, Min. Travel Time= 0.1 min Avg. Velocity = 6.45 fps, Avg. Travel Time= 0.2 min

Peak Storage= 779 cf @ 12.42 hrs Average Depth at Peak Storage= 4.00' Bank-Full Depth= 4.00' Flow Area= 12.6 sf, Capacity= 205.67 cfs

48.0" Round Pipe n= 0.011 Concrete pipe, straight & clean Length= 62.0' Slope= 0.0147 '/' Inlet Invert= 682.31', Outlet Invert= 681.40'



Summary for Reach 3R: Reservoir Brook

[91] Warning: Storage range exceeded by 0.48'
[55] Hint: Peak inflow is 140% of Manning's capacity
[63] Warning: Exceeded Reach 2R INLET depth by 0.43' @ 13.56 hrs

 Inflow Area =
 326.060 ac, 5.11% Impervious, Inflow Depth > 2.06" for 25yr event

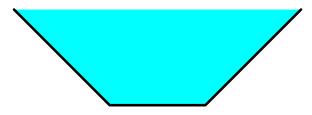
 Inflow =
 217.90 cfs @ 12.41 hrs, Volume=
 56.090 af

 Outflow =
 205.67 cfs @ 13.60 hrs, Volume=
 56.083 af, Atten= 6%, Lag= 71.5 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.23 fps, Min. Travel Time= 3.6 min Avg. Velocity = 3.02 fps, Avg. Travel Time= 11.0 min

Peak Storage= 44,171 cf @ 13.55 hrs Average Depth at Peak Storage= 3.48' Bank-Full Depth= 3.00' Flow Area= 18.0 sf, Capacity= 156.15 cfs

3.00' x 3.00' deep channel, n= 0.040 Mountain streams Side Slope Z-value= 1.0 '/' Top Width= 9.00' Length= 1,983.0' Slope= 0.0300 '/' Inlet Invert= 681.40', Outlet Invert= 622.00'



Summary for Reach 4R: Rt 104 Easterly Culvert

[52] Hint: Inlet/Outlet conditions not evaluated[62] Hint: Exceeded Reach 3R OUTLET depth by 0.23' @ 9.68 hrs

 Inflow Area =
 466.360 ac, 11.40% Impervious, Inflow Depth > 2.25" for 25yr event

 Inflow =
 434.83 cfs @ 12.51 hrs, Volume=
 87.334 af

 Outflow =
 434.80 cfs @ 12.51 hrs, Volume=
 87.334 af, Atten= 0%, Lag= 0.1 min

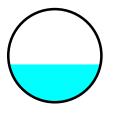
Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 40.73 fps, Min. Travel Time= 0.1 min Avg. Velocity = 11.57 fps, Avg. Travel Time= 0.2 min

Existing

Prepared by {enter your company name here} HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLC

Peak Storage= 1,324 cf @ 12.51 hrs Average Depth at Peak Storage= 2.42' Bank-Full Depth= 6.00' Flow Area= 28.3 sf, Capacity= 1,271.30 cfs

72.0" Round Pipe n= 0.011 Concrete pipe, straight & clean Length= 124.0' Slope= 0.0645 '/' Inlet Invert= 622.00', Outlet Invert= 614.00'



Summary for Reach 5R: Reservoir Brook

[91] Warning: Storage range exceeded by 1.77'
[55] Hint: Peak inflow is 216% of Manning's capacity
[62] Hint: Exceeded Reach 4R OUTLET depth by 2.37' @ 12.60 hrs

 Inflow Area =
 466.360 ac, 11.40% Impervious, Inflow Depth > 2.25" for 25yr event

 Inflow =
 434.80 cfs @
 12.51 hrs, Volume=
 87.334 af

 Outflow =
 428.01 cfs @
 12.62 hrs, Volume=
 87.326 af, Atten= 2%, Lag= 6.9 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 11.09 fps, Min. Travel Time= 3.3 min Avg. Velocity = 3.26 fps, Avg. Travel Time= 11.1 min

Peak Storage= 83,645 cf @ 12.57 hrs Average Depth at Peak Storage= 4.77' Bank-Full Depth= 3.00' Flow Area= 21.0 sf, Capacity= 201.12 cfs

4.00' x 3.00' deep channel, n= 0.040 Mountain streams Side Slope Z-value= 1.0 '/' Top Width= 10.00' Length= 2,167.0' Slope= 0.0332 '/' Inlet Invert= 614.00', Outlet Invert= 542.00'

Summary for Reach 6R: Easterly Culvert into Pond

[52] Hint: Inlet/Outlet conditions not evaluated
[55] Hint: Peak inflow is 1875% of Manning's capacity
[76] Warning: Detained 63.482 af (Pond w/culvert advised)
[62] Hint: Exceeded Reach 5R OUTLET depth by 4.46' @ 47.99 hrs

 Inflow Area =
 495.540 ac, 12.02% Impervious, Inflow Depth > 2.26" for 25yr event

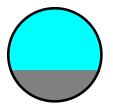
 Inflow =
 465.83 cfs @ 12.61 hrs, Volume=
 93.401 af

 Outflow =
 26.58 cfs @ 11.37 hrs, Volume=
 76.992 af, Atten= 94%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 2.49 fps, Min. Travel Time= 0.4 min Avg. Velocity = 2.14 fps, Avg. Travel Time= 0.5 min

Peak Storage= 687 cf @ 11.38 hrs Average Depth at Peak Storage= 4.50' above invert (3.00' above fill) Bank-Full Depth= 4.50' above invert (3.00' above fill) Flow Area= 11.3 sf, Capacity= 24.84 cfs

54.0" Round Pipe w/ 18.0" inside fill n= 0.025 Corrugated metal Length= 61.0' Slope= 0.0016 '/' (101 Elevation Intervals) Inlet Invert= 542.00', Outlet Invert= 541.90'



Summary for Reach 7R: Rt 104 Westerly Culvert

[52] Hint: Inlet/Outlet conditions not evaluated[55] Hint: Peak inflow is 103% of Manning's capacity[88] Warning: Qout>Qin may require smaller dt or Finer Routing

 Inflow Area =
 86.790 ac, 17.76% Impervious, Inflow Depth =
 1.88" for 25yr event

 Inflow =
 125.39 cfs @
 12.37 hrs, Volume=
 13.578 af

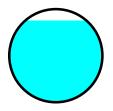
 Outflow =
 125.66 cfs @
 12.37 hrs, Volume=
 13.578 af, Atten= 0%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 19.64 fps, Min. Travel Time= 0.1 min Avg. Velocity = 6.22 fps, Avg. Travel Time= 0.3 min

Peak Storage= 788 cf @ 12.35 hrs Average Depth at Peak Storage= 2.63' Bank-Full Depth= 3.00' Flow Area= 7.1 sf, Capacity= 121.77 cfs

Existing Prepared by {enter your company name here} HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLC

36.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 120.0' Slope= 0.0333 '/' Inlet Invert= 640.00', Outlet Invert= 636.00'



Summary for Reach 8R: Central Culvert into Pond

[52] Hint: Inlet/Outlet conditions not evaluated[55] Hint: Peak inflow is 2694% of Manning's capacity[76] Warning: Detained 13.779 af (Pond w/culvert advised)

 Inflow Area =
 119.930 ac, 22.48% Impervious, Inflow Depth =
 2.17" for 25yr event

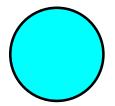
 Inflow =
 188.72 cfs @
 12.39 hrs, Volume=
 21.712 af

 Outflow =
 7.55 cfs @
 10.92 hrs, Volume=
 21.712 af, Atten= 96%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 5.72 fps, Min. Travel Time= 0.2 min Avg. Velocity = 4.88 fps, Avg. Travel Time= 0.2 min

Peak Storage= 84 cf @ 10.93 hrs Average Depth at Peak Storage= 1.33' Bank-Full Depth= 1.33' Flow Area= 1.4 sf, Capacity= 7.00 cfs

16.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 60.0' Slope= 0.0083 '/' Inlet Invert= 542.50', Outlet Invert= 542.00'



Summary for Reach 9R: Westerly culvert into Pond

[52] Hint: Inlet/Outlet conditions not evaluated[55] Hint: Peak inflow is 222% of Manning's capacity[76] Warning: Detained 1.243 af (Pond w/culvert advised)

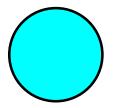
Inflow Area =	32.040 ac, 33.21% Impervious,	Inflow Depth = 2.95" for 25yr event
Inflow =	62.80 cfs @ 12.42 hrs, Volume=	= 7.864 af
Outflow =	30.21 cfs @ 12.12 hrs, Volume=	= 7.864 af, Atten= 52%, Lag= 0.0 min

Existing Prepared by {enter your company name here} HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLC

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 18.27 fps, Min. Travel Time= 0.1 min Avg. Velocity = 9.63 fps, Avg. Travel Time= 0.1 min

Peak Storage= 97 cf @ 12.13 hrs Average Depth at Peak Storage= 1.50' Bank-Full Depth= 1.50' Flow Area= 1.8 sf, Capacity= 28.33 cfs

18.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 55.0' Slope= 0.0727 '/' Inlet Invert= 544.00', Outlet Invert= 540.00'



Summary for Reach 10R: (new Reach)

[91] Warning: Storage range exceeded by 0.41' [55] Hint: Peak inflow is 146% of Manning's capacity

Inflow Are	a =	130.360 ac,	7.73% Impervious, Inflow	Depth = 2.08"	for 25yr event
Inflow	=	140.56 cfs @	12.69 hrs, Volume=	22.628 af	-
Outflow	=	140.46 cfs @	12.70 hrs, Volume=	22.628 af, Atte	en= 0%, Lag= 0.5 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 5.82 fps, Min. Travel Time= 0.3 min Avg. Velocity = 2.65 fps, Avg. Travel Time= 0.7 min

Peak Storage= 2,801 cf @ 12.69 hrs Average Depth at Peak Storage= 2.41' Bank-Full Depth= 2.00' Flow Area= 18.0 sf, Capacity= 96.39 cfs

3.00' x 2.00' deep channel, n= 0.040 Mountain streams Side Slope Z-value= 3.0 '/' Top Width= 15.00' Length= 116.0' Slope= 0.0172 '/' Inlet Invert= 546.00', Outlet Invert= 544.00'



Summary for Pond 1P: Meredith Reservoir

Inflow Area =123.050 ac,3.86% Impervious, Inflow Depth =1.93" for 25yr eventInflow =157.57 cfs @12.41 hrs, Volume=19.748 afOutflow =58.93 cfs @12.97 hrs, Volume=19.511 af, Atten= 63%, Lag= 33.3 minPrimary =58.93 cfs @12.97 hrs, Volume=19.511 af					
Routing by Stor-Ind method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 780.00' @ 12.97 hrs Surf.Area= 3.545 ac Storage= 6.939 af					
Plug-Flow detention time= 142.6 min calculated for 19.511 af (99% of inflow) Center-of-Mass det. time= 135.6 min (1,005.5 - 869.9)					
Volume Invert Avail.Storage Storage Description					
#1 778.00' 14.178 af Custom Stage Data (Prismatic) Listed below (Recalc)					
Elevation Surf.Area Inc.Store Cum.Store (feet) (acres) (acre-feet) (acre-feet)					
778.00 3.387 0.000 0.000 782.00 3.702 14.178 14.178					
Device Routing Invert Outlet Devices					
#1 Primary 778.00' 30.0" Round Culvert X 3.00 L= 30.0' CMP, square edge headwall, Ke= 0.500 Inlet / Outlet Invert= 778.00' / 777.40' S= 0.0200 '/' Cc= 0.900 n= 0.013 Corrugated PE, smooth interior, Flow Area= 4.91 sf					
Primary OutFlow Max=58.93 cfs @ 12.97 hrs HW=780.00' (Free Discharge)					
Summary for Pond 2P: Wilcom Pond					
[93] Warning: Storage range exceeded by 1.16					
Inflow Area = 39.030 ac, 12.13% Impervious, Inflow Depth = 2.25" for 25yr event Inflow = 61.11 cfs @ 12.37 hrs, Volume= 7.303 af Outflow = 40.94 cfs @ 12.37 hrs, Volume= 3.291 af, Atten= 33%, Lag= 0.0 min Primary = 5.71 cfs @ 12.37 hrs, Volume= 2.271 af Secondary = 35.23 cfs @ 12.37 hrs, Volume= 1.019 af					
Routing by Stor-Ind method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs / 2 Peak Elev= 717.16' @ 12.37 hrs Surf.Area= 0.350 ac Storage= 1.626 af					
Plug-Flow detention time= 242.2 min calculated for 3.290 af (45% of inflow) Center-of-Mass det. time= 117.9 min (975.6 - 857.8)					

Volume	Invert	Avail.Storage	Storage Description
#1	710.00'	1.626 af	Custom Stage Data (Prismatic) Listed below (Recalc)

Existing

Prepared by {enter your company name here} HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLC

Elevatio (fee 710.0	et) (acres) 00 0.192	(ac	c.Store re-feet) 0.000	Cum.Store (acre-feet) 0.000	
716.0	0 0.350		1.626	1.626	
Device	Routing	Invert	Outlet De	evices	_
#1	Primary	715.75'	1.0" x 1.0	0" Horiz. Orifice/Grate X 12.00 columns	
			X 12 row	vs C= 0.600 in 24.0" x 24.0" Grate (25% open area)	
				to weir flow at low heads	
#2	Secondary	715.95'		ng x 5.0' breadth Broad-Crested Rectangular Weir	
			Head (fee	eet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00	
			2.50 3.00	00 3.50 4.00 4.50 5.00 5.50	
			Coef. (Er	nglish) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65	
			2.67 2.66	6 2.68 2.70 2.74 2.79 2.88	
Primary	OutFlow Max=	5.71 cfs @	2 12.37 hrs	rs HW=717.16' (Free Discharge)	

1=Orifice/Grate (Orifice Controls 5.71 cfs @ 5.71 fps)

Secondary OutFlow Max=35.21 cfs @ 12.37 hrs HW=717.16' (Free Discharge) -2=Broad-Crested Rectangular Weir (Weir Controls 35.21 cfs @ 2.92 fps)

Summary for Pond 3P: Monkey Pond

[93] Warning: Storage range exceeded by 0.89'

[58] Hint: Peaked 0.89' above defined flood level

[61] Hint: Exceeded Reach 6R outlet invert by 3.91' @ 12.83 hrs

[63] Warning: Exceeded Reach 8R INLET depth by 1.97' @ 12.83 hrs

[63] Warning: Exceeded Reach 9R INLET depth by 0.31' @ 12.83 hrs

[61] Hint: Exceeded Reach 10R outlet invert by 1.81' @ 12.83 hrs

Inflow Are	a =	790.260 ac, 14.04% Impervious, Inflow Depth > 2.00" for 25yr eve	ent
Inflow	=	206.77 cfs @ 12.69 hrs, Volume= 131.687 af	
Outflow	=	202.15 cfs @ 12.83 hrs, Volume= 128.891 af, Atten= 2%, Lag=	= 8.8 min
Primary	=	202.15 cfs @ 12.83 hrs, Volume= 128.891 af	

Routing by Stor-Ind method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs / 2 Peak Elev= 545.89' @ 12.83 hrs Surf.Area= 2.000 ac Storage= 7.766 af Flood Elev= 545.00' Surf.Area= 2.000 ac Storage= 7.766 af

Plug-Flow detention time= 58.2 min calculated for 128.863 af (98% of inflow) Center-of-Mass det. time= 29.7 min (1,556.8 - 1,527.0)

Volume	Invert	Avail.Storage	Storage D	escription	
#1	540.00'	7.766 af	Custom S	tage Data	(Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Are (acres			m.Store cre-feet)	
540.00	0.86	7 0.0	000	0.000	
542.00	1.51	7 2.3	884	2.384	
544.00	1.91	0 3.4	27	5.811	
545.00	2.00	0 1.9	955	7.766	

Device	Routing	Invert	Outlet Devices		
#1	Primary	541.00'	48.0" Round RCP_Round 48"		
			L= 40.0' RCP, groove end projecting, Ke= 0.200		
			Inlet / Outlet Invert= 541.00' / 540.20' S= 0.0200 '/' Cc= 0.900		
			n= 0.011 Concrete pipe, straight & clean, Flow Area= 12.57 sf		
#2	Primary	541.00'	48.0" Round Steel Culvert		
			L= 47.0' CMP, projecting, no headwall, Ke= 0.900		
			Inlet / Outlet Invert= 541.00' / 540.00' S= 0.0213 '/' Cc= 0.900		
			n= 0.012 Steel, smooth, Flow Area= 12.57 sf		
Primarv	Primary OutFlow Max=197.56 cfs @ 12.83 hrs HW=545.79' (Free Discharge)				
·			ontrols 117.77 cfs @ 9.90 fps)		
			79.78 cfs @ 6.35 fps)		

2=Steel Culvert (Inlet Controls 79.78 cfs @ 6.35 fps)

Summary for Link 2L: Lake Waukewan

Inflow Are	a =	790.260 ac, 1	4.04% Impervious	, Inflow Depth >	1.96" f	or 25yr event
Inflow	=	202.15 cfs @	12.83 hrs, Volum	e= 128.891 a	ſ	
Primary	=	202.15 cfs @	12.83 hrs, Volum	e= 128.891 a	f, Atten	= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs

Existing
Prepared by {enter your company name here}
HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLC

Time span=2.00-48.00 hrs, dt=0.01 hrs, 4601 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Subcat 1Runoff Area=123.050 ac3.86% ImperviousRunoff Depth=2.57"Flow Length=2,832'Slope=0.1200 '/'Tc=28.2 minCN=70Runoff=213.12 cfs26.345 af
Subcatchment 2S: Subcat 2Runoff Area=203.010 ac5.87% ImperviousRunoff Depth=2.84"Flow Length=4,382'Slope=0.0950 '/'Tc=41.3 minCN=73Runoff=326.01 cfs48.071 af
Subcatchment 3S: Subcat 3Runoff Area=39.030 ac12.13% ImperviousRunoff Depth=2.93"Flow Length=2,511'Slope=0.0910 '/'Tc=26.3 minCN=74Runoff=80.49 cfs9.544 af
Subcatchment 4S: Subcat 4Runoff Area=47.760 ac22.35% ImperviousRunoff Depth=3.32"Flow Length=3,272'Slope=0.0980 '/'Tc=27.9 minCN=78Runoff=108.56 cfs13.195 af
Subcatchment 5S: Subcat 5Runoff Area=140.300 ac26.00% ImperviousRunoff Depth=3.41"Flow Length=3,798'Slope=0.0810 '/'Tc=33.5 minCN=79Runoff=302.08 cfs39.903 af
Subcatchment 6S: Subcat 6Runoff Area=130.360 ac7.73% ImperviousRunoff Depth=2.75"Flow Length=4,593'Slope=0.0810 '/'Tc=47.8 minCN=72Runoff=187.25 cfs29.871 af
Subcatchment 7S: Subcat 7Runoff Area=32.040 ac33.21% ImperviousRunoff Depth=3.71"Flow Length=3,154'Slope=0.0630 '/'Tc=29.8 minCN=82Runoff=78.85 cfs9.911 af
Subcatchment 8S: Subcat 8Runoff Area=33.140 ac34.86% ImperviousRunoff Depth=3.71"Flow Length=2,745'Slope=0.0470 '/'Tc=30.8 minCN=82Runoff=80.41 cfs10.252 af
Subcatchment 9S: Subcat 9Runoff Area=29.180 ac22.05% ImperviousRunoff Depth=3.22"Flow Length=2,576'Slope=0.0700 '/'Tc=28.0 minCN=77Runoff=64.14 cfs7.826 af
Subcatchment 10S: Subcat 10Runoff Area=12.390 ac29.64% ImperviousRunoff Depth=3.12"Flow Length=961'Slope=0.0830 '/'Tc=12.1 minCN=76Runoff=37.19 cfs3.224 af
Reach 1R: Reservoir Brook Avg. Flow Depth=1.74' Max Vel=8.13 fps Inflow=83.48 cfs 26.105 af n=0.040 L=2,356.0' S=0.0398 '/' Capacity=202.98 cfs Outflow=83.23 cfs 26.096 af
Reach 2R: Res Rd Culvert Avg. Flow Depth=4.00' Max Vel=18.65 fps Inflow=380.60 cfs 74.167 af 48.0" Round Pipe n=0.011 L=62.0' S=0.0147 '/' Capacity=205.67 cfs Outflow=215.29 cfs 74.155 af
Reach 3R: Reservoir Brook Avg. Flow Depth=3.48' Max Vel=9.23 fps Inflow=215.29 cfs 74.155 af n=0.040 L=1,983.0' S=0.0300 '/' Capacity=156.15 cfs Outflow=205.67 cfs 74.148 af
Reach 4R: Rt 104 Easterly Culvert Avg. Flow Depth=2.63' Max Vel=42.40 fps Inflow=505.95 cfs 114.051 af 72.0" Round Pipe n=0.011 L=124.0' S=0.0645 '/' Capacity=1,271.30 cfs Outflow=505.88 cfs 114.051 af
Reach 5R: Reservoir Brook Avg. Flow Depth=5.32' Max Vel=11.32 fps Inflow=505.88 cfs 114.051 af n=0.040 L=2,167.0' S=0.0332 '/' Capacity=201.12 cfs Outflow=499.65 cfs 114.043 af
Reach 6R: Easterly Culvert into PondAvg. Flow Depth=3.00'Max Vel=2.49 fpsInflow=552.24 cfs121.869 af54.0" Round Pipe w/ 18.0" inside filln=0.025L=61.0'S=0.0016 '/'Capacity=24.84 cfsOutflow=26.58 cfs78.194 af

kistingType III 24-hr 50yr Rainfrepared by {enter your company name here}Printed 12rdroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLCPrinted 12	
Pach 7R: Rt 104 Westerly Culvert Avg. Flow Depth=3.00' Max Vel=19.64 fps Inflow=145.18 cfs 36.0" Round Pipe n=0.013 L=120.0' S=0.0333 '/' Capacity=121.77 cfs Outflow=130.65 cfs	
each 8R: Central Culvert into Pond Avg. Flow Depth=1.33' Max Vel=5.72 fps Inflow=202.19 cfs 16.0" Round Pipe n=0.013 L=60.0' S=0.0083 '/' Capacity=7.00 cfs Outflow=7.54 cfs	
each 9R: Westerly culvert into Pond Avg. Flow Depth=1.50' Max Vel=18.27 fps Inflow=78.85 cfs 18.0" Round Pipe n=0.013 L=55.0' S=0.0727 '/' Capacity=28.33 cfs Outflow=30.38 cfs	
Avg. Flow Depth=2.85' Max Vel=6.11 fps Inflow=187.25 cfs n=0.040 L=116.0' S=0.0172 '/ Capacity=96.39 cfs Outflow=187.15 cfs	
Ond 1P: Meredith Reservoir Peak Elev=780.64' Storage=9.202 af Inflow=213.12 cfs 30.0" Round Culvert x 3.00 n=0.013 L=30.0' S=0.0200 '/' Outflow=83.48 cfs	
Peak Elev=717.06' Storage=1.626 af Inflow=80.49 cfs Primary=5.51 cfs 2.452 af Secondary=31.23 cfs 0.993 af Outflow=36.74 cfs	
Ond 3P: Monkey PondPeak Elev=548.04' Storage=7.766 af Inflow=255.48 cfs 2Outflow=276.99 cfs 2	
nk 2L: Lake Waukewan Inflow=276.99 cfs 2 Primary=276.99 cfs 2	
Total Runoff Area = 790.260 ac Runoff Volume = 198.143 af Average Runoff Dept	:h = 3.01"

Total Runoff Area = 790.260 ac Runoff Volume = 198.143 af Average Runoff Depth = 3.01" 85.96% Pervious = 679.318 ac 14.04% Impervious = 110.942 ac

Summary for Subcatchment 1S: Subcat 1

Runoff = 213.12 cfs @ 12.41 hrs, Volume= 26.345 af, Depth= 2.57"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50yr Rainfall=5.70"

Area	(ac)	CN	Desc	ription						
14.	820	55	Woo	Noods, Good, HSG B						
74.	760	70	Woo	ds, Good,	HSG C					
16.	140	73	Woo	ds, Fair, H	ISG C					
5.	100	61	>75%	6 Grass co	over, Good,	HSG B				
2.	070	74	>75%	6 Grass co	over, Good,	HSG C				
1.	290	79	50-75	5% Grass	cover, Fair	, HSG C				
3.	3.310 92 Urban commercial, 85% imp, HSG B									
1.	1.700 94 Urban commercial, 85% imp, HSG C									
0.580 94 Urban commercial, 85% imp, HSG C										
3.280 98 Water Surface, 0% imp, HSG A										
123.	050	70	Weig	hted Aver	age					
118.	298		96.14	1% Pervio	us Area					
4.	751		3.86%	% Impervi	ous Area					
Tc	Length		Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
28.2	2,832	2 0.	1200	1.68		Lag/CN Method,				

Summary for Subcatchment 2S: Subcat 2

Runoff = 326.01 cfs @ 12.58 hrs, Volume= 48.071 af, Depth= 2.84"

Area (ac)	CN	Description
118.170	70	Woods, Good, HSG C
37.880	73	Woods, Fair, HSG C
23.520	77	Woods, Good, HSG D
7.540	74	>75% Grass cover, Good, HSG C
0.900	79	50-75% Grass cover, Fair, HSG C
11.110	94	Urban commercial, 85% imp, HSG C
2.920	95	Urban commercial, 85% imp, HSG D
0.970	98	Water Surface, 0% imp, HSG A
203.010	73	Weighted Average
191.085		94.13% Pervious Area
11.925		5.87% Impervious Area

					name here 6 HydroCAD	e} Software Solutions LLC	<i>)</i>	Printed 12/23/2020 Page 56
	Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description		
_	41.3	4,382	0.0950	1.77	. ,	Lag/CN Method,		

Summary for Subcatchment 3S: Subcat 3

Type III 24-hr 50yr Rainfall=5.70"

80.49 cfs @ 12.36 hrs, Volume= 9.544 af, Depth= 2.93" Runoff =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50yr Rainfall=5.70"

	Area	(ac)	CN	Desc	cription		
	30.	940	70	Woo	ds, Good,	HSG C	
	2.	520	74	>75%	6 Grass co	over, Good,	I, HSG C
_	5.	570	94	Urba	in commei	cial, 85% ir	imp, HSG C
	39.	030	74	Weig	ghted Aver	age	
	34.295 87.87% Pervious Area						
	4.734 12.13% Impervious Area						
	-			~ '		A 14	
	Tc	Lengt		Slope	Velocity	Capacity	Description
	(min)	(feet	:)	(ft/ft)	(ft/sec)	(cfs)	
	26.3	2,51	1 0.	.0910	1.59		Lag/CN Method,
		-					-

Summary for Subcatchment 4S: Subcat 4

Runoff 108.56 cfs @ 12.37 hrs, Volume= 13.195 af, Depth= 3.32" =

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50yr Rainfall=5.70"

Area	(ac) C	CN	Desc	ription			
20.	230	70	Woo	ds, Good,	HSG C		
7.	890	73	Woo	ds, Fair, H	ISG C		
7.	080	77	Woo	ds, Good,	HSG D		
7.	400	94	Urba	n commei	cial, 85% ir	mp, HSG C	
2.	340	94	Urba	n commei	cial, 85% ir	mp, HSG C	
2.	820	95	Urba	n commei	cial, 85% ir	mp, HSG D	
47.	760	78	Weig	hted Aver	age		
37.	084		77.65	5% Pervio	us Area		
10.	676		22.35	5% Imperv	vious Area		
Тс	Length	S	Slope	Velocity	Capacity	Description	
(min)	(feet)		<u>(ft/ft)</u>	(ft/sec)	(cfs)		
27.9	3,272	0.0	0980	1.96		Lag/CN Method,	

Existing

Summary for Subcatchment 5S: Subcat 5

Runoff = 302.08 cfs @ 12.47 hrs, Volume= 39.903 af, Depth= 3.41"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50yr Rainfall=5.70"

Area (ac)	CN	Desc	cription					
25.990	70) Woo	ds, Good,	HSG C				
49.750	73	3 Woo	ds, Fair, H	ISG C				
8.780	77	7 Woo	ds, Good,	HSG D				
7.760	74	4 >75%	% Grass co	over, Good,	, HSG C			
0.550	79	9 50-7	5% Grass	cover, Fair	, HSG C			
4.560	80) >75%	% Grass co	over, Good,	, HSG D			
19.720	94	1 Urba	an commei	rcial, 85% i	mp, HSG C			
20.960	94	1 Urba	Urban commercial, 85% imp, HSG C					
2.230	95	5 Urba	an commei	rcial, 85% i	mp, HSG D			
140.300	79) Weig	ghted Aver	age				
103.826		74.0	0% Pervio	us Area				
36.473		26.0	0% Imperv	vious Area				
			-					
Tc Le	ngth	Slope	Velocity	Capacity	Description			
(min) (f	eet)	(ft/ft)	(ft/sec)	(cfs)				
33.5 3	,798	0.0810	1.89		Lag/CN Method,			

Summary for Subcatchment 6S: Subcat 6

Runoff = 187.25 cfs @ 12.69 hrs, Volume= 29.871 af, Depth= 2.75"

Area (ac) C	N I	Description							
10.2	260	55	Woods, Good,	HSG B						
50.4	460	70	Woods, Good,	HSG C						
49.2	240	73	Woods, Fair, H	ISG C						
5.2	250	74 :	>75% Grass c	over, Good,	, HSG C					
3.2	290	79	50-75% Grass	cover, Fair	, HSG C					
1.9	970 9	92	Urban commercial, 85% imp, HSG B							
2.5	2.560 94 Urban commercial, 85% imp, HSG C									
7.3	7.330 94 Urban commercial, 85% imp, HSG C									
130.3	360	72	Weighted Ave	rage						
120.2	279	ļ	92.27% Pervic	ous Area						
10.0	081		7.73% Impervi	ious Area						
Тс	Length	Slo	ope Velocity	Capacity	Description					
(min)	(feet)	(f	ft/ft) (ft/sec)	(cfs)						
47.8	4,593	0.08	810 1.60		Lag/CN Method,					

Summary for Subcatchment 7S: Subcat 7

Runoff = 78.85 cfs @ 12.41 hrs, Volume= 9.911 af, Depth= 3.71"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50yr Rainfall=5.70"

Area (a	ac) (CN	Desc	ription			
0.1	70	55	Woo	ds, Good,	HSG B		
1.0	60	70	Woo	ds, Good,	HSG C		
12.6	80	73	Woo	ds, Fair, H	ISG C		
5.5	10	77	Woo	ds, Good,	HSG D		
0.1	00	74	>75%	6 Grass co	over, Good,	HSG C	
5.7	30	94	Urba	n commei	rcial, 85% ir	mp, HSG C	
6.7	90	95	Urba	n commei	rcial, 85% ir	mp, HSG D	
32.0	40	82	Weig	hted Aver	age		
21.3	98		66.79	9% Pervio	us Area		
10.6	42		33.21	I% Imperv	vious Area		
Tc I	Length	S	lope	Velocity	Capacity	Description	
(min)	(feet)		(ft/ft)	(ft/sec)	(cfs)		
29.8	3,154	0.0	0630	1.77		Lag/CN Method,	

Summary for Subcatchment 8S: Subcat 8

Runoff	=	80.41 cfs @	12.42 hrs, Volume=	10.252 af, Depth= 3.71"
--------	---	-------------	--------------------	-------------------------

Area ((ac) (CN	Desc	ription			
0.2	270	70	Woo	ds, Good,	HSG C		
19.1	120	73	Woo	ds, Fair, H	ISG C		
0.1	160	77	Woo	ds, Good,	HSG D		
1.	540	94	Urba	n commei	rcial, 85% ir	mp, HSG C	
10.	730	94	Urba	n commei	rcial, 85% ir	mp, HSG C	
1.:	320	95	Urba	n commei	rcial, 85% ir	mp, HSG D	
33.	140	82	Weig	hted Aver	age		
21.	588		65.14	4% Pervio	us Area		
11.	551		34.86	5% Imperv	vious Area		
Тс	Length		lope	Velocity	Capacity	Description	
(min)	(feet)	((ft/ft)	(ft/sec)	(cfs)		
30.8	2,745	0.0	0470	1.48		Lag/CN Method,	

Summary for Subcatchment 9S: Subcat 9

Runoff = 64.14 cfs @ 12.39 hrs, Volume= 7.826 af, Depth= 3.22"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 50yr Rainfall=5.70"

 Area (a	c)	CN	Desc	ription			
1.58	30	55	Woo	ds, Good,	HSG B		
16.60	00	73	Woo	ds, Fair, H	ISG C		
1.81	10	77	Woo	ds, Good,	HSG D		
1.62	20	61	>75%	6 Grass co	over, Good,	, HSG B	
0.73	30	92	Urba	n commei	rcial, 85% ir	mp, HSG B	
6.34	40	94	Urba	n commei	rcial, 85% ir	mp, HSG C	
 0.50	00	95	Urba	n commei	rcial, 85% ir	mp, HSG D	
29.18	30	77	Weig	hted Aver	age		
22.74	45		77.95	5% Pervio	us Area		
6.43	35		22.05	5% Imperv	vious Area		
Tc L	ength	า 3	Slope	Velocity	Capacity	Description	
 (min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	-	
 28.0	2,576	S 0	.0700	1.53		Lag/CN Method,	

Summary for Subcatchment 10S: Subcat 10

Runoff	=	37.19 cfs @	12.17 hrs, Volume=	3.224 af, Depth= 3.12"
--------	---	-------------	--------------------	------------------------

Area (ac)	CN	Desc	ription			
3.5	540	55	Woo	ds, Good,	HSG B		
3.7	730	73	Woo	ds, Fair, H	ISG C		
0.1	110	61	>75%	6 Grass co	over, Good,	HSG B	
1.6	660	92	Urba	n commer	cial, 85% ir	mp, HSG B	
2.6	660	94	Urba	n commer	cial, 85% ir	mp, HSG C	
0.6	690	98	Wate	er Surface,	, 0% imp, H	ISG A	
12.3	390	76	Weig	hted Aver	age		
8.7	718		70.36	5% Pervio	us Area		
3.6	672		29.64	4% Imperv	vious Area		
Тс	Length	า 3	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
12.1	961	I 0.	.0830	1.33		Lag/CN Method,	

Summary for Reach 1R: Reservoir Brook

[79] Warning: Submerged Pond 1P Primary device # 1 OUTLET by 0.34'

 Inflow Area =
 123.050 ac,
 3.86% Impervious, Inflow Depth >
 2.55" for 50yr event

 Inflow =
 83.48 cfs @
 12.93 hrs, Volume=
 26.105 af

 Outflow =
 83.23 cfs @
 13.07 hrs, Volume=
 26.096 af, Atten= 0%, Lag= 8.7 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 8.13 fps, Min. Travel Time= 4.8 min Avg. Velocity = 2.54 fps, Avg. Travel Time= 15.5 min

Peak Storage= 24,112 cf @ 12.99 hrs Average Depth at Peak Storage= 1.74' Bank-Full Depth= 3.00' Flow Area= 19.5 sf, Capacity= 202.98 cfs

5.00' x 3.00' deep channel, n= 0.040 Mountain streams Side Slope Z-value= 0.5 '/' Top Width= 8.00' Length= 2,356.0' Slope= 0.0398 '/' Inlet Invert= 776.00', Outlet Invert= 682.31'



Summary for Reach 2R: Res Rd Culvert

[52] Hint: Inlet/Outlet conditions not evaluated

[55] Hint: Peak inflow is 185% of Manning's capacity

[76] Warning: Detained 8.042 af (Pond w/culvert advised)

[62] Hint: Exceeded Reach 1R OUTLET depth by 3.32' @ 12.30 hrs

 Inflow Area =
 326.060 ac, 5.11% Impervious, Inflow Depth > 2.73" for 50yr event

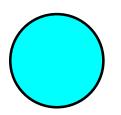
 Inflow =
 380.60 cfs @ 12.62 hrs, Volume=
 74.167 af

 Outflow =
 215.29 cfs @ 12.29 hrs, Volume=
 74.155 af, Atten= 43%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs / 2 Max. Velocity= 18.65 fps, Min. Travel Time= 0.1 min Avg. Velocity = 6.76 fps, Avg. Travel Time= 0.2 min

Peak Storage= 779 cf @ 12.30 hrs Average Depth at Peak Storage= 4.00' Bank-Full Depth= 4.00' Flow Area= 12.6 sf, Capacity= 205.67 cfs

48.0" Round Pipe n= 0.011 Concrete pipe, straight & clean Length= 62.0' Slope= 0.0147 '/' Inlet Invert= 682.31', Outlet Invert= 681.40'



Summary for Reach 3R: Reservoir Brook

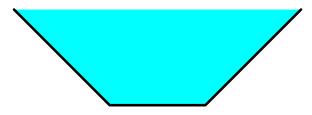
[91] Warning: Storage range exceeded by 0.48'
[55] Hint: Peak inflow is 138% of Manning's capacity
[63] Warning: Exceeded Reach 2R INLET depth by 0.43' @ 14.60 hrs

Inflow Area = 326.060 ac, 5.11% Impervious, Inflow Depth > 2.73" for 50yr event Inflow = 215.29 cfs @ 12.29 hrs, Volume= 74.155 af Outflow = 205.67 cfs @ 13.90 hrs, Volume= 74.148 af, Atten= 4%, Lag= 96.6 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.23 fps, Min. Travel Time= 3.6 min Avg. Velocity = 3.21 fps, Avg. Travel Time= 10.3 min

Peak Storage= 44,171 cf @ 13.84 hrs Average Depth at Peak Storage= 3.48' Bank-Full Depth= 3.00' Flow Area= 18.0 sf, Capacity= 156.15 cfs

3.00' x 3.00' deep channel, n= 0.040 Mountain streams Side Slope Z-value= 1.0 '/' Top Width= 9.00' Length= 1,983.0' Slope= 0.0300 '/' Inlet Invert= 681.40', Outlet Invert= 622.00'



Summary for Reach 4R: Rt 104 Easterly Culvert

[52] Hint: Inlet/Outlet conditions not evaluated[62] Hint: Exceeded Reach 3R OUTLET depth by 0.23' @ 9.08 hrs

 Inflow Area =
 466.360 ac, 11.40% Impervious, Inflow Depth > 2.93" for 50yr event

 Inflow =
 505.95 cfs @ 12.47 hrs, Volume=
 114.051 af

 Outflow =
 505.88 cfs @ 12.47 hrs, Volume=
 114.051 af, Atten= 0%, Lag= 0.1 min

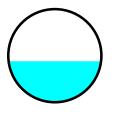
Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 42.40 fps, Min. Travel Time= 0.0 min Avg. Velocity = 12.32 fps, Avg. Travel Time= 0.2 min

Existing

Prepared by {enter your company name here} HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLC

Peak Storage= 1,480 cf @ 12.47 hrs Average Depth at Peak Storage= 2.63' Bank-Full Depth= 6.00' Flow Area= 28.3 sf, Capacity= 1,271.30 cfs

72.0" Round Pipe n= 0.011 Concrete pipe, straight & clean Length= 124.0' Slope= 0.0645 '/' Inlet Invert= 622.00', Outlet Invert= 614.00'



Summary for Reach 5R: Reservoir Brook

[91] Warning: Storage range exceeded by 2.32' [55] Hint: Peak inflow is 252% of Manning's capacity

[62] Hint: Exceeded Reach 4R OUTLET depth by 2.72' @ 12.56 hrs

Inflow Area = 466.360 ac, 11.40% Impervious, Inflow Depth > 2.93" for 50yr event Inflow = 505.88 cfs @ 12.47 hrs, Volume= 114.051 af Outflow = 499.65 cfs @ 12.58 hrs, Volume= 114.043 af, Atten= 1%, Lag= 6.6 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 11.32 fps, Min. Travel Time= 3.2 min Avg. Velocity = 3.47 fps, Avg. Travel Time= 10.4 min

Peak Storage= 95,682 cf @ 12.53 hrs Average Depth at Peak Storage= 5.32' Bank-Full Depth= 3.00' Flow Area= 21.0 sf, Capacity= 201.12 cfs

4.00' x 3.00' deep channel, n= 0.040 Mountain streams Side Slope Z-value= 1.0 '/' Top Width= 10.00' Length= 2,167.0' Slope= 0.0332 '/' Inlet Invert= 614.00', Outlet Invert= 542.00'

Summary for Reach 6R: Easterly Culvert into Pond

[52] Hint: Inlet/Outlet conditions not evaluated

[55] Hint: Peak inflow is 2223% of Manning's capacity [76] Warning: Detained 89.705 af (Pond w/culvert advised)

[62] Hint: Exceeded Reach 5R OUTLET depth by 4.46' @ 47.99 hrs

 Inflow Area =
 495.540 ac, 12.02% Impervious, Inflow Depth > 2.95" for 50yr event

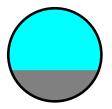
 Inflow =
 552.24 cfs @ 12.56 hrs, Volume=
 121.869 af

 Outflow =
 26.58 cfs @ 10.86 hrs, Volume=
 78.194 af, Atten= 95%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 2.49 fps, Min. Travel Time= 0.4 min Avg. Velocity = 2.14 fps, Avg. Travel Time= 0.5 min

Peak Storage= 687 cf @ 10.87 hrs Average Depth at Peak Storage= 4.50' above invert (3.00' above fill) Bank-Full Depth= 4.50' above invert (3.00' above fill) Flow Area= 11.3 sf, Capacity= 24.84 cfs

54.0" Round Pipe w/ 18.0" inside fill n= 0.025 Corrugated metal Length= 61.0' Slope= 0.0016 '/' (101 Elevation Intervals) Inlet Invert= 542.00', Outlet Invert= 541.90'



Summary for Reach 7R: Rt 104 Westerly Culvert

[52] Hint: Inlet/Outlet conditions not evaluated[55] Hint: Peak inflow is 119% of Manning's capacity[76] Warning: Detained 0.309 af (Pond w/culvert advised)

 Inflow Area =
 86.790 ac, 17.76% Impervious, Inflow Depth =
 2.30" for 50yr event

 Inflow =
 145.18 cfs @
 12.37 hrs, Volume=
 16.640 af

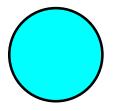
 Outflow =
 130.65 cfs @
 12.29 hrs, Volume=
 16.640 af, Atten=

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 19.64 fps, Min. Travel Time= 0.1 min Avg. Velocity = 6.50 fps, Avg. Travel Time= 0.3 min

Peak Storage= 848 cf @ 12.30 hrs Average Depth at Peak Storage= 3.00' Bank-Full Depth= 3.00' Flow Area= 7.1 sf, Capacity= 121.77 cfs

Existing Prepared by {enter your company name here} HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLC

36.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 120.0' Slope= 0.0333 '/' Inlet Invert= 640.00', Outlet Invert= 636.00'



Summary for Reach 8R: Central Culvert into Pond

[52] Hint: Inlet/Outlet conditions not evaluated[55] Hint: Peak inflow is 2887% of Manning's capacity[76] Warning: Detained 18.230 af (Pond w/culvert advised)

 Inflow Area =
 119.930 ac, 22.48% Impervious, Inflow Depth =
 2.69" for 50yr event

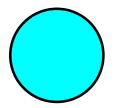
 Inflow =
 202.19 cfs @
 12.42 hrs, Volume=
 26.892 af

 Outflow =
 7.54 cfs @
 10.42 hrs, Volume=
 22.452 af, Atten= 96%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 5.72 fps, Min. Travel Time= 0.2 min Avg. Velocity = 4.88 fps, Avg. Travel Time= 0.2 min

Peak Storage= 84 cf @ 10.43 hrs Average Depth at Peak Storage= 1.33' Bank-Full Depth= 1.33' Flow Area= 1.4 sf, Capacity= 7.00 cfs

16.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 60.0' Slope= 0.0083 '/' Inlet Invert= 542.50', Outlet Invert= 542.00'



Summary for Reach 9R: Westerly culvert into Pond

- [52] Hint: Inlet/Outlet conditions not evaluated
- [55] Hint: Peak inflow is 278% of Manning's capacity
- [76] Warning: Detained 2.062 af (Pond w/culvert advised)
- [85] Warning: Oscillations may require smaller dt or Finer Routing (severity=2)

Existing Prepared by {enter your company name here} HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLC

Type III 24-hr 50yr Rainfall=5.70" Printed 12/23/2020 Page 65

 Inflow Area =
 32.040 ac, 33.21% Impervious, Inflow Depth =
 3.71" for 50yr event

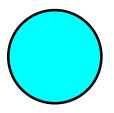
 Inflow =
 78.85 cfs @
 12.41 hrs, Volume=
 9.911 af

 Outflow =
 30.38 cfs @
 12.06 hrs, Volume=
 9.911 af, Atten= 61%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 18.27 fps, Min. Travel Time= 0.1 min Avg. Velocity = 10.10 fps, Avg. Travel Time= 0.1 min

Peak Storage= 97 cf @ 12.07 hrs Average Depth at Peak Storage= 1.50' Bank-Full Depth= 1.50' Flow Area= 1.8 sf, Capacity= 28.33 cfs

18.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 55.0' Slope= 0.0727 '/' Inlet Invert= 544.00', Outlet Invert= 540.00'



Summary for Reach 10R: (new Reach)

[91] Warning: Storage range exceeded by 0.85' [55] Hint: Peak inflow is 194% of Manning's capacity

 Inflow Area =
 130.360 ac,
 7.73% Impervious, Inflow Depth =
 2.75"
 for 50yr event

 Inflow =
 187.25 cfs @
 12.69 hrs, Volume=
 29.871 af

 Outflow =
 187.15 cfs @
 12.70 hrs, Volume=
 29.871 af, Atten= 0%, Lag= 0.5 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 6.11 fps, Min. Travel Time= 0.3 min Avg. Velocity = 2.81 fps, Avg. Travel Time= 0.7 min

Peak Storage= 3,556 cf @ 12.69 hrs Average Depth at Peak Storage= 2.85' Bank-Full Depth= 2.00' Flow Area= 18.0 sf, Capacity= 96.39 cfs

3.00' x 2.00' deep channel, n= 0.040 Mountain streams Side Slope Z-value= 3.0 '/' Top Width= 15.00' Length= 116.0' Slope= 0.0172 '/' Inlet Invert= 546.00', Outlet Invert= 544.00'

‡

Summary for Pond 1P: Meredith Reservoir

Inflow = 213.1 Outflow = 83.4	48 cfs @ 12.93 hrs, Volume= 2	epth = 2.57" for 50yr event 26.345 af 26.105 af, Atten= 61%, Lag= 31.2 min 26.105 af							
	Routing by Stor-Ind method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 780.64' @ 12.93 hrs Surf.Area= 3.595 ac Storage= 9.202 af								
	me= 126.3 min calculated for 26.105 af me= 120.9 min (982.3 - 861.4)	i (99% of inflow)							
Volume Invert	Avail.Storage Storage Description								
#1 778.00'	14.178 af Custom Stage Data (Prismatic) Listed below (Recalc)							
Elevation Surf.Are (feet) (acre	rea Inc.Store Cum.Store	,							
778.00 3.38									
782.00 3.70	702 14.178 14.178								
Device Routing	Invert Outlet Devices								
#1 Primary	778.00' 30.0" Round Culvert X 3 L= 30.0' CMP, square ed								
		00' / 777.40' = 0.0200'/ Cc= 0.900							
		, smooth interior, Flow Area= 4.91 sf							
Primary OutFlow Max=83.48 cfs @ 12.93 hrs HW=780.64' (Free Discharge) ↓1=Culvert (Inlet Controls 83.48 cfs @ 5.67 fps)									
Summary for Pond 2P: Wilcom Pond									
[93] Warning: Storage range exceeded by 1.06'									
Inflow = 80.4 Outflow = 36.7 Primary = 5.5		epth = 2.93" for 50yr event 9.544 af 3.445 af, Atten= 54%, Lag= 0.0 min 2.452 af 0.993 af							

Routing by Stor-Ind method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs / 2 Peak Elev= 717.06' @ 12.36 hrs Surf.Area= 0.350 ac Storage= 1.626 af

Plug-Flow detention time= 274.8 min calculated for 3.445 af (36% of inflow) Center-of-Mass det. time= 146.0 min (996.0 - 850.0)

Volume	Invert	Avail.Storage	Storage Description
#1	710.00'	1.626 af	Custom Stage Data (Prismatic) Listed below (Recalc)

Existing

Prepared by {enter your company name here} HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLC

Elevatio (fee			c.Store re-feet)	Cum.Store (acre-feet)			
710.0	0.192		0.000	0.000			
716.0	0 0.350		1.626	1.626			
Device	Routing	Invert	Outlet De	evices			
#1	Primary	715.75'	1.0" x 1.0	0" Horiz. Orifice/Grate X 12.00 columns			
	·		X 12 rows	/s C= 0.600 in 24.0" x 24.0" Grate (25% open area)			
			Limited to	o weir flow at low heads			
#2	Secondary	715.95'	10.0' long	g x 5.0' breadth Broad-Crested Rectangular Weir			
	-		Head (fee	et) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00			
			2.50 3.00 3.50 4.00 4.50 5.00 5.50				
			Coef. (En	nglish) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65			
			2.67 2.66	6 2.68 2.70 2.74 2.79 2.88			
Primary OutFlow Max = 5.51 cfs @ 12.36 brs HW = 717.06' (Free Discharge)							

Primary OutFlow Max=5.51 cfs @ 12.36 hrs HW=717.06' (Free Discharge) **1=Orifice/Grate** (Orifice Controls 5.51 cfs @ 5.51 fps)

Secondary OutFlow Max=31.17 cfs @ 12.36 hrs HW=717.06' (Free Discharge) -2=Broad-Crested Rectangular Weir (Weir Controls 31.17 cfs @ 2.81 fps)

Summary for Pond 3P: Monkey Pond

[93] Warning: Storage range exceeded by 3.04'

[58] Hint: Peaked 3.04' above defined flood level

[88] Warning: Qout>Qin may require smaller dt or Finer Routing

[63] Warning: Exceeded Reach 6R INLET depth by 1.54' @ 12.65 hrs

[63] Warning: Exceeded Reach 8R INLET depth by 4.20' @ 12.65 hrs

[63] Warning: Exceeded Reach 9R INLET depth by 2.54' @ 12.65 hrs

[62] Hint: Exceeded Reach 10R OUTLET depth by 1.20' @ 12.65 hrs

Inflow Area =	790.260 ac,	14.04% Impervious, Inflow	v Depth > 2.18" for 50yr event
Inflow =	255.48 cfs @	12.65 hrs, Volume=	143.653 af
Outflow =	276.99 cfs @	12.65 hrs, Volume=	141.649 af, Atten= 0%, Lag= 0.0 min
Primary =	276.99 cfs @	12.65 hrs, Volume=	141.649 af

Routing by Stor-Ind method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs / 2 Peak Elev= 548.04' @ 12.65 hrs Surf.Area= 2.000 ac Storage= 7.766 af Flood Elev= 545.00' Surf.Area= 2.000 ac Storage= 7.766 af

Plug-Flow detention time= 40.6 min calculated for 141.618 af (99% of inflow) Center-of-Mass det. time= 21.0 min (1,494.3 - 1,473.3)

Volume	Invert	Avail.Storage	Storage Description
#1	540.00'	7.766 af	Custom Stage Data (Prismatic) Listed below (Recalc)

Existing

Prepared by {enter your company name here} HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLC

Elevatio (fee		rf.Area (acres)		c.Store re-feet)			
540.0	00	0.867		0.000	0 0.000		
542.0	00	1.517		2.384	4 2.384		
544.0	00	1.910		3.427	7 5.811		
545.0)0	2.000		1.955	5 7.766		
Device	Routing		Invert		et Devices		
#1	Primary		541.00'		' Round RCP_Round 48"		
#2	Primary		541.00'	L= 40.0' RCP, groove end projecting, Ke= 0.200 Inlet / Outlet Invert= $541.00' / 540.20'$ S= $0.0200' / Cc= 0.900$ n= 0.011 Concrete pipe, straight & clean, Flow Area= 12.57 sf 48.0" Round Steel Culvert L= $47.0'$ CMP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= $541.00' / 540.00'$ S= $0.0213' / Cc= 0.900$ n= 0.012 Steel, smooth, Flow Area= 12.57 sf			
				~			

Primary OutFlow Max=276.96 cfs @ 12.65 hrs HW=548.04' (Free Discharge) -1=RCP_Round 48" (Inlet Controls 169.75 cfs @ 13.51 fps) -2=Steel Culvert (Inlet Controls 107.21 cfs @ 8.53 fps)

Summary for Link 2L: Lake Waukewan

Inflow Area	a =	790.260 ac, 14.04% Impervious,	Inflow Depth > 2.15"	for 50yr event
Inflow	=	276.99 cfs @ 12.65 hrs, Volume	= 141.649 af	
Primary	=	276.99 cfs @ 12.65 hrs, Volume	 = 141.649 af, Atte 	en= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs

Existing
Prepared by {enter your company name here}
HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLC

Time span=2.00-48.00 hrs, dt=0.01 hrs, 4601 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: Subcat 1Runoff Area=123.050 ac3.86% ImperviousRunoff Depth=3.36"Flow Length=2,832'Slope=0.1200 '/'Tc=28.2 minCN=70Runoff=281.01 cfs34.478 af
Subcatchment 2S: Subcat 2Runoff Area=203.010 ac5.87% ImperviousRunoff Depth=3.67"Flow Length=4,382'Slope=0.0950 '/'Tc=41.3 minCN=73Runoff=422.59 cfs62.077 af
Subcatchment 3S: Subcat 3Runoff Area=39.030 ac12.13% ImperviousRunoff Depth=3.77"Flow Length=2,511'Slope=0.0910 '/'Tc=26.3 minCN=74Runoff=103.78 cfs12.272 af
Subcatchment 4S: Subcat 4Runoff Area=47.760 ac22.35% ImperviousRunoff Depth=4.19"Flow Length=3,272'Slope=0.0980 '/'Tc=27.9 minCN=78Runoff=137.17 cfs16.695 af
Subcatchment 5S: Subcat 5Runoff Area=140.300 ac26.00% ImperviousRunoff Depth=4.30"Flow Length=3,798'Slope=0.0810 '/'Tc=33.5 minCN=79Runoff=379.49 cfs50.293 af
Subcatchment 6S: Subcat 6Runoff Area=130.360 ac7.73% ImperviousRunoff Depth=3.57"Flow Length=4,593'Slope=0.0810 '/'Tc=47.8 minCN=72Runoff=243.81 cfs38.743 af
Subcatchment 7S: Subcat 7Runoff Area=32.040 ac33.21% ImperviousRunoff Depth=4.63"Flow Length=3,154'Slope=0.0630 '/'Tc=29.8 minCN=82Runoff=97.70 cfs12.352 af
Subcatchment 8S: Subcat 8Runoff Area=33.140 ac34.86% ImperviousRunoff Depth=4.63"Flow Length=2,745'Slope=0.0470 '/'Tc=30.8 minCN=82Runoff=99.67 cfs12.776 af
Subcatchment 9S: Subcat 9Runoff Area=29.180 ac 22.05% Impervious Runoff Depth=4.09"Flow Length=2,576'Slope=0.0700 '/' Tc=28.0 min CN=77 Runoff=81.41 cfs 9.941 af
Subcatchment 10S: Subcat 10Runoff Area=12.390 ac29.64% ImperviousRunoff Depth=3.98"Flow Length=961'Slope=0.0830 '/'Tc=12.1 minCN=76Runoff=47.42 cfs4.112 af
Reach 1R: Reservoir Brook Avg. Flow Depth=2.02' Max Vel=8.71 fps Inflow=105.77 cfs 34.235 af n=0.040 L=2,356.0' S=0.0398 '/' Capacity=202.98 cfs Outflow=105.56 cfs 34.225 af
Reach 2R: Res Rd Culvert Avg. Flow Depth=4.00' Max Vel=18.66 fps Inflow=502.44 cfs 96.303 af 48.0" Round Pipe n=0.011 L=62.0' S=0.0147 '/' Capacity=205.67 cfs Outflow=219.17 cfs 96.300 af
Reach 3R: Reservoir Brook Avg. Flow Depth=3.48' Max Vel=9.23 fps Inflow=219.17 cfs 96.300 af n=0.040 L=1,983.0' S=0.0300 '/' Capacity=156.15 cfs Outflow=205.67 cfs 96.293 af
Reach 4R: Rt 104 Easterly Culvert Avg. Flow Depth=2.86' Max Vel=44.03 fps Inflow=584.86 cfs 146.586 af 72.0" Round Pipe n=0.011 L=124.0' S=0.0645 '/' Capacity=1,271.30 cfs Outflow=584.79 cfs 146.586 af
Reach 5R: Reservoir Brook Avg. Flow Depth=5.94' Max Vel=11.51 fps Inflow=584.79 cfs 146.586 af n=0.040 L=2,167.0' S=0.0332 '/' Capacity=201.12 cfs Outflow=578.74 cfs 146.577 af
Reach 6R: Easterly Culvert into PondAvg. Flow Depth=3.00'Max Vel=2.49 fpsInflow=647.47 cfs156.519 af54.0" Round Pipe w/ 18.0" inside fill n=0.025L=61.0'S=0.0016 '/'Capacity=24.84 cfsOutflow=26.57 cfs79.449 af

Existing Prepared by {enter your company name here} HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD	
	w Depth=3.00' Max Vel=19.64 fps Inflow=249.93 cfs 30.551 af S=0.0333 '/' Capacity=121.77 cfs Outflow=127.49 cfs 30.551 af
•	ow Depth=1.33' Max Vel=5.72 fps Inflow=221.44 cfs 43.327 af .0' S=0.0083 '/' Capacity=7.00 cfs Outflow=7.56 cfs 22.809 af
	ow Depth=1.50' Max Vel=18.27 fps Inflow=97.70 cfs 12.352 af S=0.0727 '/' Capacity=28.33 cfs Outflow=29.96 cfs 12.352 af
	ow Depth=3.37' Max Vel=6.32 fps Inflow=243.81 cfs 38.743 af S=0.0172 '/' Capacity=96.39 cfs Outflow=243.71 cfs 38.743 af
	k Elev=781.48' Storage=12.246 af Inflow=281.01 cfs 34.478 af 00 n=0.013 L=30.0' S=0.0200 '/' Outflow=105.77 cfs 34.235 af
	eak Elev=718.44' Storage=1.626 af Inflow=103.78 cfs 12.272 af Secondary=105.06 cfs 10.252 af Outflow=112.96 cfs 13.856 af
Pond 3P: Monkey Pond Pea	k Elev=547.43' Storage=7.766 af Inflow=314.57 cfs 157.465 af Outflow=256.85 cfs 150.594 af
Link 2L: Lake Waukewan	Inflow=256.85 cfs 150.594 af Primary=256.85 cfs 150.594 af

Total Runoff Area = 790.260 acRunoff Volume = 253.739 afAverage Runoff Depth = 3.85"85.96% Pervious = 679.318 ac14.04% Impervious = 110.942 ac

Summary for Subcatchment 1S: Subcat 1

Runoff = 281.01 cfs @ 12.39 hrs, Volume= 34.478 af, Depth= 3.36"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100yr Rainfall=6.69"

Area (a	ac) C	N Des	cription						
14.8	20 5	55 Wo	Woods, Good, HSG B						
74.7	60 7	'0 Wo	ods, Good,	HSG C					
16.1	40 7	'3 Wo	ods, Fair, F	ISG C					
5.1	00 6	61 >75	% Grass co	over, Good	, HSG B				
2.0	70 7	'4 >75	% Grass co	over, Good	, HSG C				
1.2	90 7	' 9 50-7	75% Grass	cover, Fair	, HSG C				
3.3	510 S	02 Urb	an comme	rcial, 85% i	mp, HSG B				
1.7	00 9				mp, HSG C				
0.5	80 9	04 Urb	an comme	rcial, 85% i	mp, HSG C				
3.2	80 9	98 Wat	er Surface	<u>, 0% imp, F</u>	ISG A				
123.0	50 7	′0 Wei	ghted Avei	age					
118.2	98	96.1	4% Pervio	us Area					
4.7	51	3.86	% Impervi	ous Area					
Тс	Length	Slope	Velocity	Capacity	Description				
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)					
28.2	2,832	0.1200	1.68		Lag/CN Method,				
					-				

Summary for Subcatchment 2S: Subcat 2

Runoff = 422.59 cfs @ 12.57 hrs, Volume= 62.077 af, Depth= 3.67"

Area (ac)	CN	Description
118.170	70	Woods, Good, HSG C
37.880	73	Woods, Fair, HSG C
23.520	77	Woods, Good, HSG D
7.540	74	>75% Grass cover, Good, HSG C
0.900	79	50-75% Grass cover, Fair, HSG C
11.110	94	Urban commercial, 85% imp, HSG C
2.920	95	Urban commercial, 85% imp, HSG D
0.970	98	Water Surface, 0% imp, HSG A
203.010	73	Weighted Average
191.085		94.13% Pervious Area
11.925		5.87% Impervious Area

Existing	Тур
Prepared by {enter your company name here}	
HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLC	

Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)	·	
41.3	4,382	0.0950	1.77		Lag/CN Method,	

Summary for Subcatchment 3S: Subcat 3

Runoff = 103.78 cfs @ 12.36 hrs, Volume= 12.272 af, Depth= 3.77"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100yr Rainfall=6.69"

_	Area ((ac)	CN	Desc	cription		
	30.9	940	70	Woo	ds, Good,	HSG C	
	2.	520	74	>75%	% Grass co	over, Good,	I, HSG C
_	5.	570	94	Urba	ın commei	<u>cial, 85% i</u> r	imp, HSG C
	39.	030	74	Weig	ghted Aver	age	
	34.	295		87.8	7% Pervio	us Area	
	4.	734		12.13	3% Imperv	vious Area	
	-			0	N / I · · ·	o	
	Tc	Lengt		Slope	Velocity	Capacity	Description
_	(min)	(feet	:)	(ft/ft)	(ft/sec)	(cfs)	
	26.3	2,51	1 0	.0910	1.59		Lag/CN Method,
							-

Summary for Subcatchment 4S: Subcat 4

Runoff =	137.17 cfs @	12.37 hrs,	Volume=	16.695 af, Depth= 4.	19"
----------	--------------	------------	---------	----------------------	-----

Area ((ac) (CN	Desc	cription			
20.2	230	70	Woo	ds, Good,	HSG C		
7.8	890	73	Woo	ds, Fair, ⊢	ISG C		
7.0	080	77	Woo	ds, Good,	HSG D		
7.4	400	94	Urba	in commei	rcial, 85% ir	mp, HSG C	
2.3	340	94	Urba	in commei	rcial, 85% ir	mp, HSG C	
2.8	820	95	Urba	in commei	rcial, 85% ir	mp, HSG D	
47.	760	78	Weig	ghted Aver	age		
37.0	084		77.6	5% Pervio	us Area		
10.0	676		22.3	5% Imper\	ious Area/		
		_					
Tc	Length		ope	Velocity	Capacity	Description	
<u>(min)</u>	(feet)	(ft/ft)	(ft/sec)	(cfs)		
27.9	3,272	0.0	980	1.96		Lag/CN Method,	

Summary for Subcatchment 5S: Subcat 5

Runoff = 379.49 cfs @ 12.47 hrs, Volume= 50.293 af, Depth= 4.30"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100yr Rainfall=6.69"

Area (a	c) C	N D	Description			
25.99	90 7	70 V	Noods, Good,	HSG C		
49.75	50 7	73 V	Noods, Fair, H	ISG C		
8.78	30 7	77 V	Noods, Good,	HSG D		
7.76	50 7	74 >	>75% Grass c	over, Good	, HSG C	
0.55	50 7	79 5	50-75% Grass	cover, Fair	, HSG C	
4.56	50 B	30 >	>75% Grass c	over, Good	, HSG D	
19.72	20 9	94 L	Jrban comme	rcial, 85% i	mp, HSG C	
20.96	50 9	94 L	Jrban comme	rcial, 85% i	mp, HSG C	
2.23	30 9	95 L	Jrban comme	rcial, 85% i	mp, HSG D	
140.30	00 7	79 V	Veighted Ave	rage		
103.82	26	7	4.00% Pervic	ous Area		
36.47	73	2	26.00% Imper	vious Area		
			-			
Tc L	ength	Slo	pe Velocity	Capacity	Description	
(min)	(feet)	(ft	/ft) (ft/sec)	(cfs)		
33.5	3,798	0.08	310 1.89		Lag/CN Method,	

Summary for Subcatchment 6S: Subcat 6

Runoff = 243.81 cfs @ 12.69 hrs, Volume= 38.743 af, Depth= 3.57"

Area ((ac)	CN	Desci	ription			
10.2	260	55	Wood	ds, Good,	HSG B		
50.4	460	70	Wood	ds, Good,	HSG C		
49.2	240	73	Wood	ds, Fair, H	ISG C		
5.2	250	74	>75%	Grass co	over, Good,	, HSG C	
3.2	290	79	50-75	% Grass	cover, Fair	, HSG C	
1.9	970	92	Urbar	n commei	rcial, 85% ii	mp, HSG B	
2.	560	94	Urbar	n commei	rcial, 85% ii	mp, HSG C	
7.3	330	94	Urbar	n commei	rcial, 85% ii	mp, HSG C	
130.3	360	72	Weig	hted Aver	age		
120.2	279		92.27	% Pervio	us Area		
10.0	081		7.73%	6 Impervi	ous Area		
Tc	Length	า 5	Slope	Velocity	Capacity	Description	
(min)	(feet))	(ft/ft)	(ft/sec)	(cfs)		
47.8	4,593	3 0.	.0810	1.60		Lag/CN Method,	

Summary for Subcatchment 7S: Subcat 7

Runoff = 97.70 cfs @ 12.41 hrs, Volume= 12.352 af, Depth= 4.63"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100yr Rainfall=6.69"

Area (ac) CN	Description
0.170 55	Woods, Good, HSG B
1.060 70	Woods, Good, HSG C
12.680 73	Woods, Fair, HSG C
5.510 77	Woods, Good, HSG D
0.100 74	>75% Grass cover, Good, HSG C
5.730 94	Urban commercial, 85% imp, HSG C
6.790 95	Urban commercial, 85% imp, HSG D
32.040 82	Weighted Average
21.398	66.79% Pervious Area
10.642	33.21% Impervious Area
Tc Length S	lope Velocity Capacity Description
(min) (feet)	(ft/ft) (ft/sec) (cfs)
29.8 3,154 0.0	D630 1.77 Lag/CN Method,

Summary for Subcatchment 8S: Subcat 8

Runoff	=	99.67 cfs @	12.42 hrs, Volume=	12.776 af, Depth= 4.63"
--------	---	-------------	--------------------	-------------------------

Area (a	c) Cl	N Des	cription			
0.27	70 7	0 Wo	ods, Good,	HSG C		
19.12	20 7	'3 Wo	ods, Fair, F	ISG C		
0.16	60 7	7 Wo	ods, Good,	HSG D		
1.54	40 9	4 Urb	an comme	rcial, 85% ii	mp, HSG C	
10.73	30 9	4 Urb	an comme	rcial, 85% ii	mp, HSG C	
1.32	20 9	5 Urb	an comme	rcial, 85% ii	mp, HSG D	
33.14	40 8	2 Wei	ghted Avei	age		
21.58	38	65.1	4% Pervio	us Area		
11.55	51	34.8	6% Imper	ious Area		
	_ength	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
30.8	2,745	0.0470	1.48		Lag/CN Method,	

Summary for Subcatchment 9S: Subcat 9

Runoff = 81.41 cfs @ 12.38 hrs, Volume= 9.941 af, Depth= 4.09"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Type III 24-hr 100yr Rainfall=6.69"

Area (ac)	CN	Descri	ption			
1.580	55	Woods	s, Good,	HSG B		
16.600	73	Woods	s, Fair, H	SG C		
1.810	77	Woods	s, Good,	HSG D		
1.620	61	>75% (Grass co	over, Good,	HSG B	
0.730	92	Urban	commer	cial, 85% ir	mp, HSG B	
6.340	94	Urban	commer	cial, 85% ir	mp, HSG C	
0.500	95	Urban	commer	cial, 85% ir	mp, HSG D	
29.180	77	Weight	ted Aver	age		
22.745		77.95%	6 Pervio	us Area		
6.435		22.05%	6 Imperv	vious Area		
Tc Len	gth	Slope V	/elocity	Capacity	Description	
(min) (fe	et)	(ft/ft)	(ft/sec)	(cfs)		
28.0 2,5	576 0	.0700	1.53		Lag/CN Method,	

Summary for Subcatchment 10S: Subcat 10

Runoff	=	47.42 cfs @	12.17 hrs, Volume=	4.112 af, Depth= 3.98"
--------	---	-------------	--------------------	------------------------

Area (a	ac) C	N D	escription			
3.5	540 క	55 W	oods, Good,	HSG B		
3.7	30 7	73 W	oods, Fair, H	ISG C		
0.1	10 6	51 >7	'5% Grass c	over, Good,	, HSG B	
1.6	60 9	92 U	ban comme	rcial, 85% i	mp, HSG B	
2.6	60 9	94 U	ban comme	rcial, 85% i	mp, HSG C	
0.6	690 <u>9</u>	98 W	ater Surface	e, 0% imp, H	ISG A	
12.3	390 7	76 W	eighted Ave	rage		
8.7	'18	70	.36% Pervic	ous Area		
3.6	672	29	.64% Imper	vious Area		
Тс	Length	Slop	e Velocity	Capacity	Description	
(min)	(feet)	(ft/1	t) (ft/sec)	(cfs)		
12.1	961	0.083	0 1.33		Lag/CN Method,	

Summary for Reach 1R: Reservoir Brook

[79] Warning: Submerged Pond 1P Primary device # 1 INLET by 0.02'

 Inflow Area =
 123.050 ac,
 3.86% Impervious, Inflow Depth >
 3.34" for 100yr event

 Inflow =
 105.77 cfs @
 12.93 hrs, Volume=
 34.235 af

 Outflow =
 105.56 cfs @
 13.07 hrs, Volume=
 34.225 af, Atten= 0%, Lag= 8.1 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 8.71 fps, Min. Travel Time= 4.5 min Avg. Velocity = 2.71 fps, Avg. Travel Time= 14.5 min

Peak Storage= 28,559 cf @ 12.99 hrs Average Depth at Peak Storage= 2.02' Bank-Full Depth= 3.00' Flow Area= 19.5 sf, Capacity= 202.98 cfs

5.00' x 3.00' deep channel, n= 0.040 Mountain streams Side Slope Z-value= 0.5 '/' Top Width= 8.00' Length= 2,356.0' Slope= 0.0398 '/' Inlet Invert= 776.00', Outlet Invert= 682.31'



Summary for Reach 2R: Res Rd Culvert

[52] Hint: Inlet/Outlet conditions not evaluated

[55] Hint: Peak inflow is 244% of Manning's capacity

[76] Warning: Detained 16.506 af (Pond w/culvert advised)

[62] Hint: Exceeded Reach 1R OUTLET depth by 3.21' @ 12.22 hrs

 Inflow Area =
 326.060 ac,
 5.11% Impervious, Inflow Depth >
 3.54" for 100yr event

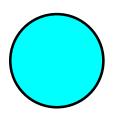
 Inflow =
 502.44 cfs @
 12.62 hrs, Volume=
 96.303 af

 Outflow =
 219.17 cfs @
 12.21 hrs, Volume=
 96.300 af, Atten= 56%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs / 2 Max. Velocity= 18.66 fps, Min. Travel Time= 0.1 min Avg. Velocity = 7.09 fps, Avg. Travel Time= 0.1 min

Peak Storage= 779 cf @ 12.22 hrs Average Depth at Peak Storage= 4.00' Bank-Full Depth= 4.00' Flow Area= 12.6 sf, Capacity= 205.67 cfs

48.0" Round Pipe n= 0.011 Concrete pipe, straight & clean Length= 62.0' Slope= 0.0147 '/' Inlet Invert= 682.31', Outlet Invert= 681.40'



Summary for Reach 3R: Reservoir Brook

[91] Warning: Storage range exceeded by 0.48'
[55] Hint: Peak inflow is 140% of Manning's capacity
[63] Warning: Exceeded Reach 2R INLET depth by 0.77' @ 15.94 hrs

 Inflow Area =
 326.060 ac, 5.11% Impervious, Inflow Depth > 3.54" for 100yr event

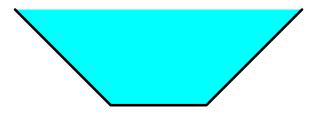
 Inflow =
 219.17 cfs @ 12.21 hrs, Volume=
 96.300 af

 Outflow =
 205.67 cfs @ 13.81 hrs, Volume=
 96.293 af, Atten= 6%, Lag= 96.1 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 9.23 fps, Min. Travel Time= 3.6 min Avg. Velocity = 3.42 fps, Avg. Travel Time= 9.7 min

Peak Storage= 44,171 cf @ 13.75 hrs Average Depth at Peak Storage= 3.48' Bank-Full Depth= 3.00' Flow Area= 18.0 sf, Capacity= 156.15 cfs

3.00' x 3.00' deep channel, n= 0.040 Mountain streams Side Slope Z-value= 1.0 '/' Top Width= 9.00' Length= 1,983.0' Slope= 0.0300 '/' Inlet Invert= 681.40', Outlet Invert= 622.00'



Summary for Reach 4R: Rt 104 Easterly Culvert

[52] Hint: Inlet/Outlet conditions not evaluated[62] Hint: Exceeded Reach 3R OUTLET depth by 0.22' @ 8.43 hrs

Inflow Area = 466.360 ac, 11.40% Impervious, Inflow Depth > 3.77" for 100yr event Inflow = 584.86 cfs @ 12.47 hrs, Volume= 146.586 af Outflow = 584.79 cfs @ 12.47 hrs, Volume= 146.586 af, Atten= 0%, Lag= 0.0 min

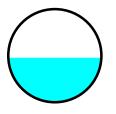
Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 44.03 fps, Min. Travel Time= 0.0 min Avg. Velocity = 13.11 fps, Avg. Travel Time= 0.2 min

Existing

Prepared by {enter your company name here} HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLC

Peak Storage= 1,647 cf @ 12.47 hrs Average Depth at Peak Storage= 2.86' Bank-Full Depth= 6.00' Flow Area= 28.3 sf, Capacity= 1,271.30 cfs

72.0" Round Pipe n= 0.011 Concrete pipe, straight & clean Length= 124.0' Slope= 0.0645 '/' Inlet Invert= 622.00', Outlet Invert= 614.00'



Summary for Reach 5R: Reservoir Brook

[91] Warning: Storage range exceeded by 2.94'

[55] Hint: Peak inflow is 291% of Manning's capacity

[62] Hint: Exceeded Reach 4R OUTLET depth by 3.10' @ 12.53 hrs

Inflow Area = 466.360 ac, 11.40% Impervious, Inflow Depth > 3.77" for 100yr event Inflow = 584.79 cfs @ 12.47 hrs, Volume= 146.586 af Outflow = 578.74 cfs @ 12.56 hrs, Volume= 146.577 af, Atten= 1%, Lag= 5.7 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 11.51 fps, Min. Travel Time= 3.1 min Avg. Velocity = 3.70 fps, Avg. Travel Time= 9.8 min

Peak Storage= 108,974 cf @ 12.51 hrs Average Depth at Peak Storage= 5.94' Bank-Full Depth= 3.00' Flow Area= 21.0 sf, Capacity= 201.12 cfs

4.00' x 3.00' deep channel, n= 0.040 Mountain streams Side Slope Z-value= 1.0 '/' Top Width= 10.00' Length= 2,167.0' Slope= 0.0332 '/' Inlet Invert= 614.00', Outlet Invert= 542.00'

Summary for Reach 6R: Easterly Culvert into Pond

[52] Hint: Inlet/Outlet conditions not evaluated

[55] Hint: Peak inflow is 2607% of Manning's capacity

[76] Warning: Detained 122.639 af (Pond w/culvert advised)

[62] Hint: Exceeded Reach 5R OUTLET depth by 4.46' @ 47.99 hrs

 Inflow Area =
 495.540 ac, 12.02% Impervious, Inflow Depth > 3.79" for 100yr event

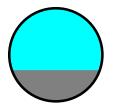
 Inflow =
 647.47 cfs @ 12.54 hrs, Volume=
 156.519 af

 Outflow =
 26.57 cfs @ 10.30 hrs, Volume=
 79.449 af, Atten= 96%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 2.49 fps, Min. Travel Time= 0.4 min Avg. Velocity = 2.13 fps, Avg. Travel Time= 0.5 min

Peak Storage= 687 cf @ 10.31 hrs Average Depth at Peak Storage= 4.50' above invert (3.00' above fill) Bank-Full Depth= 4.50' above invert (3.00' above fill) Flow Area= 11.3 sf, Capacity= 24.84 cfs

54.0" Round Pipe w/ 18.0" inside fill n= 0.025 Corrugated metal Length= 61.0' Slope= 0.0016 '/' (101 Elevation Intervals) Inlet Invert= 542.00', Outlet Invert= 541.90'



Summary for Reach 7R: Rt 104 Westerly Culvert

[52] Hint: Inlet/Outlet conditions not evaluated[55] Hint: Peak inflow is 205% of Manning's capacity[76] Warning: Detained 4.260 af (Pond w/culvert advised)

 Inflow Area =
 86.790 ac, 17.76% Impervious, Inflow Depth =
 4.22" for 100yr event

 Inflow =
 249.93 cfs @
 12.37 hrs, Volume=
 30.551 af

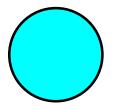
 Outflow =
 127.49 cfs @
 12.11 hrs, Volume=
 30.551 af, Atten= 49%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 19.64 fps, Min. Travel Time= 0.1 min Avg. Velocity = 7.20 fps, Avg. Travel Time= 0.3 min

Peak Storage= 848 cf @ 12.12 hrs Average Depth at Peak Storage= 3.00' Bank-Full Depth= 3.00' Flow Area= 7.1 sf, Capacity= 121.77 cfs

Existing Prepared by {enter your company name here} HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLC

36.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 120.0' Slope= 0.0333 '/' Inlet Invert= 640.00', Outlet Invert= 636.00'



Summary for Reach 8R: Central Culvert into Pond

[52] Hint: Inlet/Outlet conditions not evaluated[55] Hint: Peak inflow is 3161% of Manning's capacity[76] Warning: Detained 34.117 af (Pond w/culvert advised)

 Inflow Area =
 119.930 ac, 22.48% Impervious, Inflow Depth =
 4.34" for 100yr event

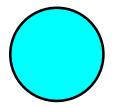
 Inflow =
 221.44 cfs @
 12.42 hrs, Volume=
 43.327 af

 Outflow =
 7.56 cfs @
 9.83 hrs, Volume=
 22.809 af, Atten= 97%, Lag= 0.0 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 5.72 fps, Min. Travel Time= 0.2 min Avg. Velocity = 4.88 fps, Avg. Travel Time= 0.2 min

Peak Storage= 84 cf @ 9.84 hrs Average Depth at Peak Storage= 1.33' Bank-Full Depth= 1.33' Flow Area= 1.4 sf, Capacity= 7.00 cfs

16.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 60.0' Slope= 0.0083 '/' Inlet Invert= 542.50', Outlet Invert= 542.00'



Summary for Reach 9R: Westerly culvert into Pond

[52] Hint: Inlet/Outlet conditions not evaluated[55] Hint: Peak inflow is 345% of Manning's capacity[76] Warning: Detained 3.116 af (Pond w/culvert advised)

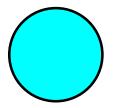
Inflow Area	a =	32.040 ac, 33.21% Impervious, Inflow Depth = 4.63" for 100yr event
Inflow	=	97.70 cfs @ 12.41 hrs, Volume= 12.352 af
Outflow	=	29.96 cfs @ 11.99 hrs, Volume= 12.352 af, Atten= 69%, Lag= 0.0 min

Existing Prepared by {enter your company name here} HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLC

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 18.27 fps, Min. Travel Time= 0.1 min Avg. Velocity = 10.57 fps, Avg. Travel Time= 0.1 min

Peak Storage= 97 cf @ 12.00 hrs Average Depth at Peak Storage= 1.50' Bank-Full Depth= 1.50' Flow Area= 1.8 sf, Capacity= 28.33 cfs

18.0" Round Pipe n= 0.013 Corrugated PE, smooth interior Length= 55.0' Slope= 0.0727 '/' Inlet Invert= 544.00', Outlet Invert= 540.00'



Summary for Reach 10R: (new Reach)

[91] Warning: Storage range exceeded by 1.37' [55] Hint: Peak inflow is 253% of Manning's capacity

Inflow Are	a =	130.360 ac,	7.73% Impervious, Inflow	Depth = 3.57"	for 100yr event
Inflow	=	243.81 cfs @	12.69 hrs, Volume=	38.743 af	-
Outflow	=	243.71 cfs @	12.69 hrs, Volume=	38.743 af, Atte	n= 0%, Lag= 0.1 min

Routing by Stor-Ind+Trans method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Max. Velocity= 6.32 fps, Min. Travel Time= 0.3 min Avg. Velocity = 2.97 fps, Avg. Travel Time= 0.7 min

Peak Storage= 4,471 cf @ 12.69 hrs Average Depth at Peak Storage= 3.37' Bank-Full Depth= 2.00' Flow Area= 18.0 sf, Capacity= 96.39 cfs

3.00' x 2.00' deep channel, n= 0.040 Mountain streams Side Slope Z-value= 3.0 '/' Top Width= 15.00' Length= 116.0' Slope= 0.0172 '/' Inlet Invert= 546.00', Outlet Invert= 544.00'



Summary for Pond 1P: Meredith Reservoir

Inflow Area =123.050 ac,3.86% Impervious, Inflow Depth =3.36"for 100yr eventInflow =281.01 cfs @12.39 hrs, Volume=34.478 afOutflow =105.77 cfs @12.93 hrs, Volume=34.235 af, Atten= 62%, Lag= 32.7 minPrimary =105.77 cfs @12.93 hrs, Volume=34.235 af				
Routing by Stor-Ind method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs Peak Elev= 781.48' @ 12.93 hrs Surf.Area= 3.661 ac Storage= 12.246 af				
Plug-Flow detention time= 115.8 min calculated for 34.227 af (99% of inflow) Center-of-Mass det. time= 111.8 min (965.4 - 853.6)				
Volume Invert Avail.Storage Storage Description				
#1 778.00' 14.178 af Custom Stage Data (Prismatic) Listed below (Recalc)				
Elevation Surf.Area Inc.Store Cum.Store (feet) (acres) (acre-feet) (acre-feet)				
778.00 3.387 0.000 0.000 782.00 3.702 14.178 14.178				
Device Routing Invert Outlet Devices #1 Primary 778.00' 30.0" Round Culvert X 3.00				
#1 Primary 778.00' 30.0" Round Culvert X 3.00 L= 30.0' CMP, square edge headwall, Ke= 0.500				
Inlet / Outlet Invert= 778.00' / 777.40' S= 0.0200 '/' Cc= 0.900				
n= 0.013 Corrugated PE, smooth interior, Flow Area= 4.91 sf				
Primary OutFlow Max=105.77 cfs @ 12.93 hrs HW=781.48' (Free Discharge) ↓1=Culvert (Inlet Controls 105.77 cfs @ 7.18 fps)				
Summary for Pond 2P: Wilcom Pond				
[93] Warning: Storage range exceeded by 2.44' [88] Warning: Qout>Qin may require smaller dt or Finer Routing				
Inflow Area = 39.030 ac, 12.13% Impervious, Inflow Depth = 3.77" for 100yr event Inflow = 103.78 cfs @ 12.36 hrs, Volume= 12.272 af				
Outflow = 112.96 cfs @ 12.36 hrs, Volume= 13.856 af, Atten= 0%, Lag= 0.0 min				
Primary = 7.90 cfs @ 12.36 hrs, Volume= 3.604 af Secondary = 105.06 cfs @ 12.36 hrs, Volume= 10.252 af				
Routing by Stor-Ind method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs / 2 Peak Elev= 718.44' @ 12.36 hrs Surf.Area= 0.350 ac Storage= 1.626 af				
Plug-Flow detention time= (not calculated: outflow precedes inflow) Center-of-Mass det. time= 29.1 min (871.8 - 842.8)				
Volume Invert Avail.Storage Storage Description				

#1	710.00'	1.626 af	Custom Stage Data (Prismatic) Listed below (Recalc)
----	---------	----------	---

Existing

Prepared by {enter your company name here} HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLC

Elevatio			c.Store re-feet)	Cum.Store (acre-feet)	
710.0	0.192		0.000	0.000	
716.0	0 0.350		1.626	1.626	
Device	Routing	Invert	Outlet De	vices	
#1	Primary	715.75'	1.0" x 1.0	" Horiz. Orific	e/Grate X 12.00 columns
			X 12 rows	s C= 0.600 in 2	24.0" x 24.0" Grate (25% open area)
			Limited to	weir flow at lo	ow heads
#2	Secondary	715.95'	10.0' long	y x 5.0' bread	h Broad-Crested Rectangular Weir
			Head (fee	et) 0.20 0.40	0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00
			2.50 3.00	3.50 4.00 4	.50 5.00 5.50
			Coef. (En	glish) 2.34 2	50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65
			2.67 2.66	5 2.68 2.70 2	.74 2.79 2.88
	• • •				<i>/</i>

Primary OutFlow Max=7.90 cfs @ 12.36 hrs HW=718.44' (Free Discharge) ←1=Orifice/Grate (Orifice Controls 7.90 cfs @ 7.90 fps)

Secondary OutFlow Max=105.02 cfs @ 12.36 hrs HW=718.44' (Free Discharge) -2=Broad-Crested Rectangular Weir (Weir Controls 105.02 cfs @ 4.21 fps)

Summary for Pond 3P: Monkey Pond

[93] Warning: Storage range exceeded by 2.43'

[58] Hint: Peaked 2.43' above defined flood level

[63] Warning: Exceeded Reach 6R INLET depth by 0.93' @ 12.65 hrs

[63] Warning: Exceeded Reach 8R INLET depth by 3.59' @ 12.65 hrs

[63] Warning: Exceeded Reach 9R INLET depth by 1.93' @ 12.65 hrs

[62] Hint: Exceeded Reach 10R OUTLET depth by 0.05' @ 12.61 hrs

Inflow Area	a =	790.260 ac, 14.04% Impervious, Inflow Depth > 2.39" for 100yr event
Inflow	=	314.57 cfs @ 12.65 hrs, Volume= 157.465 af
Outflow	=	256.85 cfs @ 12.65 hrs, Volume= 150.594 af, Atten= 18%, Lag= 0.0 min
Primary	=	256.85 cfs @ 12.65 hrs, Volume= 150.594 af

Routing by Stor-Ind method, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs / 2 Peak Elev= 547.43' @ 12.65 hrs Surf.Area= 2.000 ac Storage= 7.766 af Flood Elev= 545.00' Surf.Area= 2.000 ac Storage= 7.766 af

Plug-Flow detention time= 105.2 min calculated for 150.562 af (96% of inflow) Center-of-Mass det. time= 42.0 min (1,457.4 - 1,415.5)

Volume	Invert	Avail.Storage	Storage Descripti	on
#1	540.00'	7.766 af	Custom Stage Da	ta (Prismatic) Listed below (Recalc)
Elevation (feet)	Surf.Are (acre			-
540.00	0.86	67 0.0	0.00)
542.00	1.51	17 2.3	384 2.384	4
544.00 545.00	1.9 [,] 2.00		427 5.81 [°] 955 7.760	-

Prepared by {enter your company name here} HydroCAD® 10.00-18 s/n 00761 © 2016 HydroCAD Software Solutions LLC

Device	Routing	Invert	Outlet Devices			
#1	Primary	541.00'	48.0" Round RCP_Round 48"			
			L= 40.0' RCP, groove end projecting, Ke= 0.200			
			Inlet / Outlet Invert= 541.00' / 540.20' S= 0.0200 '/' Cc= 0.900			
			n= 0.011 Concrete pipe, straight & clean, Flow Area= 12.57 sf			
#2	Primary	541.00'	48.0" Round Steel Culvert			
			L= 47.0' CMP, projecting, no headwall, Ke= 0.900			
			Inlet / Outlet Invert= 541.00' / 540.00' S= 0.0213 '/' Cc= 0.900			
			n= 0.012 Steel, smooth, Flow Area= 12.57 sf			
· · ·	Primary OutFlow Max=256.85 cfs @ 12.65 hrs HW=547.43' (Free Discharge)					
		·	ontrols 156.36 cfs @ 12.44 fps)			
1 2 64	and Cultrart	(Inlat Controla	100.40 efc $@ 9.00$ fpc)			

2=Steel Culvert (Inlet Controls 100.49 cfs @ 8.00 fps)

Summary for Link 2L: Lake Waukewan

Inflow Are	ea =	790.260 ac, 14.04% Impervious, Inflow Depth >	2.29"	for 100yr event
Inflow	=	256.85 cfs @ 12.65 hrs, Volume= 150.594	af	
Primary	=	256.85 cfs @ 12.65 hrs, Volume= 150.594	af, Atte	en= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 2.00-48.00 hrs, dt= 0.01 hrs

Appendix D



MEMORANDUM 226315

TO:Nick Sceggell, Project FileFROM:Charlotte Brodie, Grace GlynnSUBJECT:Monkey Pond, Meredith, NH; Wetlands EvaluationDATE:December 2, 2020

1. The Lake Winnipesaukee Association has requested that DuBois & King, Inc. assist with a study of the Reservoir Brook and Monkey Pond subwatershed, in an effort to better understand the processes of excessive stormwater loading from land development into Lake Waukewan and further downstream. A part of this study is a wetlands evaluation of Monkey Pond. DuBois & King, Inc. used the New Hampshire Method for Inventorying and Evaluating Wetlands in New Hampshire (2015) to assess the functions of values of the wetland within the Pond. The results of this evaluation are presented below.

Monkey Pond is located between the railroad track along the shore of Lake Waukewan and Waukewan Road, in the Town of Meredith, as shown on the Location Map. A small Town parking lot for access to the Lake Waukewan swimming area is located at the eastern extreme, and a residence is located at the western extreme. The pond is located within the Reservoir Brook subwatershed. The total area is approximately 1.75 acres.

- 2. Field Naturalists Grace Glynn and Charlotte Brodie visited the site on July 21, 2020 to document the wetland's characteristics for the purposes of a New Hampshire Method Wetland Evaluation. The area was found to include two small, shallow, vegetated ponds as well as aquatic bed, emergent and scrubshrub wetland. The entire area would be considered jurisdictional wetland under the New Hampshire Wetland Rules. Photos of the wetland are attached.
- 3. The eastern end of the wetland is primarily scrub-shrub, with speckled alder, willow, elderberry, sweet gale, silky dogwood, highbush blueberry, meadowsweet, and buttonbush. Minor components of forested wetland occur around the edges, with common species including American elm and red maple. Towards the interior, the wetland is primarily herbaceous, with cattails, royal fern, Joe pye-weed, cinnamon fern, rice cutgrass, jewelweed, arrowhead, pickerelweed, burreed, watershield, and yellow pond lily. The wetland gives way to two small areas of open-water pond.
- 4. Reservoir Brook enters the wetland through a 6' metal "squash pipe" at the eastern end, where it deposits significant quantities of sand and mud from the upstream watershed. The stream continues as a recognizable channel for

approximately 100' before fanning out into shallow marsh and pond. Water exits Monkey Pond through two four-foot culverts under the railroad.

- 5. Three 18" culverts drain from the wetland on the south side of Waukewan Road into Monkey Pond, where they scour out plunge pools. The location of the culverts is shown in the attached exhibit.
- 6. A riparian corridor and wetland on the south side of Waukewan Road at the western end of Monkey Pond empties into the pond through a culvert.
- 7. According to the NH Natural Heritage Bureau's Data Check Tool, no rare, threatened or endangered species or significant natural communities are known in the vicinity of Monkey Pond, and none were observed during the course of field work.
- 8. The wetland as a whole was evaluated in accordance with the "Method for Inventorying and Evaluating Freshwater Wetlands in New Hampshire," as updated in 2015. The attached Data Forms document the evaluation for each of twelve wetland functions. The scores for each function are provided in Table 1 below. The scores for each category are on a scale of 1-10, with the exception of Flood Storage, which is based on a calculation, and Noteworthiness, which is ranked out of 70 possible points. With the exception of these two functions, scores from 8-10 indicate a higher performance for that function, and scores below 5 indicate that the wetland is compromised for that function (NHDES, 2015).

Wetland Function	Score
Ecological Integrity	5.0
Wetland Wildlife Habitat	3.6
Fish & Aquatic Habitat	3.1
Scenic Quality	7.7
Educational Potential	3.7
Wetland-Based Recreation	4.4
Floodwater Storage	1.8
Groundwater	2.8
Sediment Trapping	4.1
Nutrient Transformation	5.2
Shoreline Anchoring	7.5
Noteworthiness	10.0

9. **Ecological Integrity** is the overall health and stability of the wetland ecosystem. Monkey Pond received an Ecological Integrity score of 5. Though the wetland itself has not been directly impacted by human activity and includes only a small percentage of non-native invasive plants, development and human activity in the surrounding area—including impervious surfaces and commercial development—diminish the ecological integrity of the site.

Monkey Pond's Ecological Integrity score contributed to its lower **Wetland Wildlife Habitat** score (3.6) and **Fish & Aquatic Habitat** score (3.1). These low scores can be attributed in part to land use in the above watershed, where commercial development appears to be causing high sedimentation levels in the wetland and its stream channel. Photos of sediment deposition in the wetland are attached. These lower scoring wetland functions can indicate potential focus areas for restoration and mitigation. For example, reducing sedimentation and erosion upstream and managing invasive species will help maintain open water habitat in the wetland and its stream channel, increasing habitat value for fish and waterfowl. Though culverts are present, access to the wetland by wildlife may be limited by the road and railroad.

The wetland's **Scenic Quality** score was 7.7. This high score is unsurprising considering that the site offers striking contrasts between Lake Waukewan and a diversity of wetland vegetation types. An area of open water in the wetland is visible from the road, and nearby landowners report watching ducks feeding there.

The wetland's **Educational Potential** score was 3.7. This low score is attributed to lack of access to the wetland. Students would need to walk along a busy road to access the wetland itself. The wetland received low **Recreation** score (4.4) for a similar reason: no trails are present and access is limited.

The wetland's **Flood Storage** score of 1.8 indicates Low Flood Value. This is likely because the wetland is small in size. However, despite its small size, the wetland exhibits features indicating a fluctuation in water level of up to three feet. This fluctuation indicates that significant flood storage occurs within the overall wetland, even though the site is relatively small.

The wetland received a **Groundwater** score of 2.8. This was Monkey Pond's lowest-scoring function, likely due to a low percentage of highly permeable soil types and absence of public wellhead protection area. However, it should be noted that Lake Waukewan is the public drinking water supply for the town of Meredith, and conservation of wetlands along the lake edge should be prioritized accordingly.

The wetland received a **Sediment Trapping** score of 4.1. This low score is attributed to the wetland's outlets into Lake Waukewan, which cause sediment to enter the lake. However, because of the wetland's linear shape and small culverts, it should be noted that it is likely functioning as a sediment trap to some degree. This is especially true in the wetland's densely vegetated areas.

Though the wetland only received a score of 5.2 for **Nutrient Transformation**, this lower score is likely due to the wetland's small size and subsequently low flood storage score.

A score of 7.5 for **Shoreline Anchoring** indicates that Monkey Pond contributes to the stabilization of Lake Waukewan's shoreline. A diversity of wetland vegetation types is present with moderate vegetation cover and a moderate diversity of substrate, preventing erosion into the lake. Because the wetland is semi-permanently or permanently flooded and contains abundant organic soils, it also helps to prevent excess nutrients and pollutants from entering Lake Waukewan. The wetland received a score of 10 for **Noteworthiness** due to its location within an area of Highest Ranked Habitat as identified on the NH Wildlife Action Plan. Wildlife that utilize Lake Waukewan as habitat can also find food and cover at Monkey Pond.

10. Conclusions

General:

Although degraded by sedimentation, Monkey Pond supports a good diversity of native wetland vegetation and performs a variety of wetland functions at fair levels.

Regarding Monkey Pond's contribution to the water quality of Lake Waukewan:

Areas of dense wetland vegetation, including cattails and alders, sequester nutrients, preventing pollutants from entering Lake Waukewan and helping to maintain water quality in the Lake.

Regarding the question of dredging:

As mentioned above, areas of dense vegetation help to sequester nutrients in the substrate. Therefore, disturbance of natural vegetation in the wetland should be avoided. Dredging and other activities that disturb organic wetland soils and root systems are not recommended, and are unlikely to be permittable through the NHDES Wetlands Bureau.

Regarding candidacy for wetland enhancement:

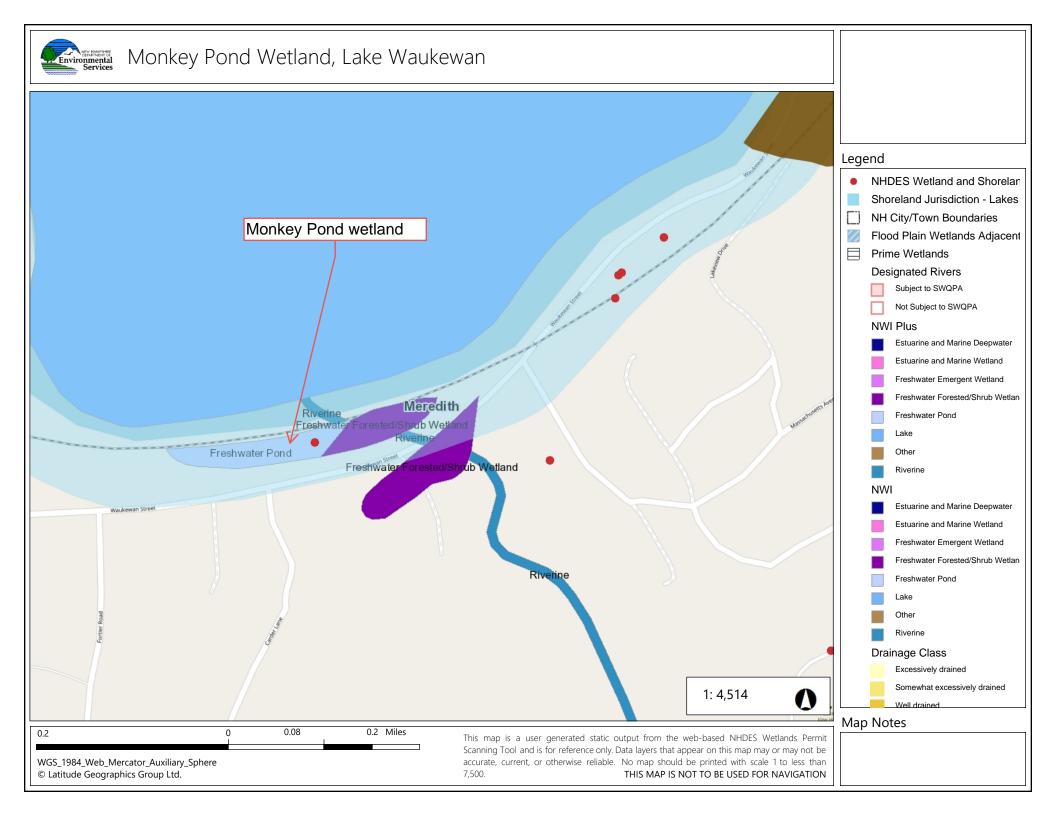
Although degraded by sedimentation from the upstream watershed, the wetland is still supporting a good diversity of native wetland species, and is performing a variety of functions, including the provision of some wildlife and waterfowl habitat. Since dredging is not recommended, since there is little to no room to increase protective buffer at the wetland itself, and since the vegetative community is well adapted to the environment and stable, the wetland itself is not a good candidate for wetland enhancement.

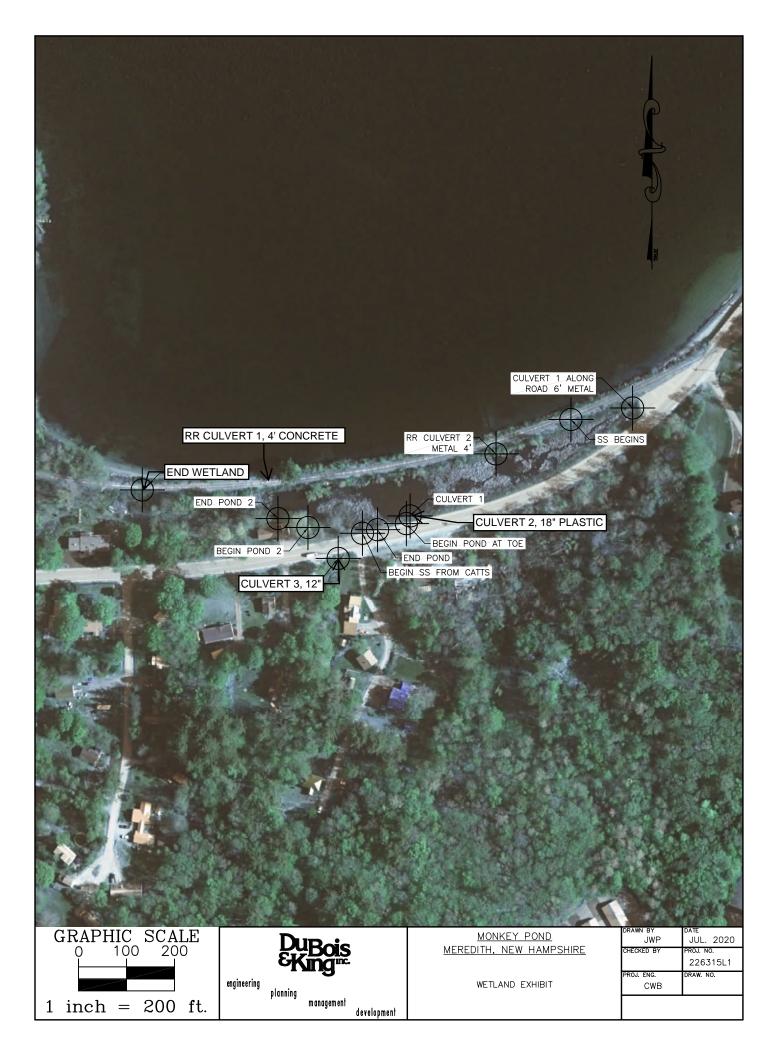
Recommendations for management:

Monkey Pond's ecological integrity, its ability to function as wildlife, fish and aquatic habitat, and its flood storage potential have been compromised by sedimentation from upstream sources. It is recommended that restoration and conservation efforts be focused upstream, where land use practices have significant effects on the ecological integrity of Monkey Pond and Lake Waukewan. For example, improved stormwater management at or below development sites and restoration of riparian buffers would help to reduce erosion and sedimentation and provide shade and cover for aquatic organisms.

Use of the NH Method Evaluation:

The NH Method Evaluation of Monkey Pond's functions and values can be considered a baseline against which to measure changes resulting from future development or restoration in the watershed above.





Wetland Name/Code: Monkey Pond

_ Evaluation Date: July 21, 2020

Evaluator: Charlotte Brodie, Grace Glynn

1 – ECOLOGICAL INTEGRITY

	Evaluation Questions	Observations & Notes	Answers	Score
1.	Are there land uses in the wetland's watershed that could degrade water quality in the wetland?	Commercial development upstream may be causing high sedimentation levels in wetland	 a. Less than 5% of the watershed has land uses that could degrade water quality. b. 5-10% of the watershed has land uses that could degrade water quality. c. > 10% of the watershed has land uses that could degrade water quality. 	10 5 1
2.	Is there evidence of fill in the wetland?	None observed	a. Less than 1 %b. From 1-3 %c. More than 3 %	10 5 1
3.	What percentage of the wetland has been altered by agricultural activities?	None	a. Less than 5 %b. From 5 to 25 %c. More than 25 %	10 5 1
4.	What percentage of the wetland has been adversely impacted by logging activity within the last 10 years?	None	a. Less than 1%b. From 1 to 10 %c. More than 10 %	10 5 1
5.	How much human activity is taking place in the wetland (e.g. ATV use, trails, cars, dumping of brush and garbage, etc.)?	None	 a. Low: Few trails in use, little or no traffic, and little or no litter. b. Moderate: Some used trails, roads, litter c. High: Many trails, roads, and/or litter 	10 5 1
6.	What percentage of the wetland is occupied by invasive plant species?	Purple loosestrife and Japanese knotweed present along wetland/ upland edge	 a. None b. 1-5% of the wetland has invasive species c. > 5% of the wetland has invasive species 	10 5 1
7.	Are there roads, driveways and/or railroads crossing or adjacent to the wetland or come within 500 ft. of the wetland?	The wetland is bordered by a railroad on one side and Waukewan Street on the other	 a. No roads, driveways or railroads. within 500 ft. of, or in the wetland b. Roads, driveways, railroads are within 500 ft of the wetland c. Roads, driveways, railroads cross, or are adjacent to, the wetland 	10 5 1
8.	How much human activity is taking place in the upland within 500 feet of the wetland edge?	Residential and recreational use along road and railroad	 a. Less than 5% or no activity b. Human activity evident in up to 25% of the 500 ft zone c. Human activity evident in more than 25% of the 500 ft zone 	10 5 1
9.	What is the percent of impervious surface within 500 feet of the wetland edge?	Small parking lot, rail- road, and paved road adjacent to wetland	 a. Less than 3% impervious area within 500 ft of the wetland edge b. 3-10% impervious area within 500 ft of the wetland edge c. Greater than 10% impervious area within 500 ft of the wetland edge 	10 5 1
10	. Is there a human-made structure that regulates the flow of water through the wetland?	There are three 18" culverts beneath Waukewan St, 2 4' culverts beneath the railroad, and one 6' culvert carrying Reservoir Brook beneath Waukewan Street	 a. No human made structures present upstream of, or in the wetland. b. One or more human made structures present upstream of, or in the wetland but hydrologic modification is slight c. One or more human made structures present upstream of, or in the wetland that severely block or alter surface water hydrology 	10 5 1

Monkey Pond

Wetland Name/Code:_

___ Evaluation Date: July 21, 2020

Evaluator: Glynn

2 – WETLAND-DEPENDENT WILDLIFE HABITAT

	Evaluation Questions	Observations & Notes	Answers	Score
1.	What is the wetland acreage (including upland islands)?	1.75 acres	a. More than 100 acresb. From 20 - 100 acresc. Less than 20 acres	10 5 1
2.	What is the score for Ecological Integrity?	5	Average score for Ecological Integrity	5
3.	Has water quality in the wetland been degraded by land use in the watershed?	Likely degraded by commercial development upstream (to the south of the site)	Record Answer from Ecological Integrity, Question 1	1
4.	What is the area of shallow permanent open water less than 6.6 feet deep, including streams and shallow ponds that are part of the wetland complex?	<.5 acres	a. More than 3 acresb. From 0.5 to 3 acresc. Less than 0.5 acre	10 5 1
5.	Is there deepwater habitat (lakes or ponds > 6.6ft deep) and/or 4 th order or higher rivers associated with the wetland?	No	 a. Deepwater stream ≥1 mile long and/or lake or pond ≥10 acres present b. Deepwater stream < 1 mile long and/or lake or pond < 10 acres present c. No deepwater stream, lake or pond present 	10 5 1
6.	What is the diversity of vegetation classes in the wetland? Refer to Appendix F for more information about wetland vegetation classes.	3 classes: PUB, PSS, PEM	 a. Three or more wetland classes (including upland islands) present b. Two wetland classes (including upland islands) present c. One wetland class present 	10 5 1
7.	Are other wetlands in close proximity to the study wetland?	Yes, Wetland B is of similar size and directly south of Waukewan St.	 a. Other connected or unconnected wetlands within a 0.25 mile distance b. Wetland connected to other wetlands within a 0.5 to 1 mile distance by perennial stream or lake, OR other unconnected wetlands are present within a 0.25 to 0.5 mile distance c. Wetland not hydrologically connected to other wetlands within 1 mile and more than 0.5 miles from other unconnected wetlands. 	10 5 1

Wetland Name/Code: <u>Monkey Pond</u>

Evaluation Date: July 21, 2020

Evaluator: Charlotte Brodie, Grace

2 - WETLAND-DEPENDENT WILDLIFE HABITAT (continued)

	Evaluation Questions	Observations & Notes	Answers	Score
8.	Are there wildlife travel corridors allowing access to other wetlands?	Movement likely blocked by railroad and road	 a. Free access along well vegetated stream corridor, woodland, or lakeshore b. Access partially blocked by roads, urban areas, or other obstructions c. Access blocked by roads, urban areas, or other obstructions 	10 5 1
9.	What percentage of the wetland edge is bordered by undisturbed woodland or idle land (e.g. shrub land or abandoned fields) at least 500 feet in width?	Estimated 10% undisturbed border (on western bound- ary)	 a. More than 95% of the wetland b. More than 75-95% of the wetland c. Less than 75% of the wetland 	10 5 1
10.	What percentage of the wetland is occupied by invasive plant species?		Record Answer from Ecological Integrity, Question 6	5

AVERAGE SCORE FOR WILDLIFE HABITAT

(Add scores for each question and divide by 10)

3.6

Wetland Name/Code: Monkey Pond

_____ Evaluation Date: July 21, 2020 _____ Evaluator: CB, GG

3 – FISH AND AQUATIC LIFE HABITAT

	Evaluation Questions	Observations & Notes	Answers	Score
1.	What is the dominant land use in the watershed above wetland?	Mostly wooded with scat- tered commercial develop- ment	 a. Woodland, wetland, or abandoned farmland b. Active farmland or rural residential c. Urban and heavily developed suburban areas, commercial and industrial areas. 	10 5 1
2.	Has water quality in the wetland been degraded by land use in the watershed?		Record Answer from Ecological Integrity , Question 1	_1
3.	What is the area of <u>shallow</u> permanent open water less than 6.6 ft deep, including streams and ponds within the wetland?		Record Answer from Wetland-Dependent Wildlife Habitat, Question 4	1
4.	What is the acreage of <u>deepwater</u> habitats deeper than 6.6 feet (pond or lake) associated with the wetland?	Two small open-water ponds present	 a. More than 100 acres b. From 10 to 100 acres c. Less than 10 acres d. deepwater pond or lake not present 	10 5 1 0
5.	What is the width (bank to bank) of the stream within the wetland?	Est. 5 ft width	 a. More than 50 feet b. From 25 to 50 feet c. Less than 25 feet d. No stream present 	10 5 1 0
6.	Does the stream channel appear to have been recently altered?		 a. Stream is in a natural channel, either a meandering low gradient stream, OR a steeper gradient stream with pools and riffles b. Portions of stream appear recently modified, OR stream formerly channelized but has regained some natural channel features c. Stream appears to have been recently been channelized, OR stream is confined in a non-vegetated chute or pipe d. No stream present 	10 5 1 0
7.	Within the wetland, what is the diversity of substrate types in the area(s) <u>occupied</u> <u>by open water</u> (flowing or standing) for the non-growing season?	gravel, muck, large rocks	 a. 4 or more substrate types b. 2 or 3 substrate types c. 1 substrate type 	10 5 1
8.	How abundant are coarse woody material and large rocks associated with the open water portion of the wetland?	Some woody material and large rocks present in stream channel	 a. Moderately Abundant to Abundant: More than 10% of the open water portion of the wetland area contains cover objects such as logs, stumps, branches and rocks b. Scarce: Less than 10% of the water open water portion of the wetland wetland area contains cover objects c. No visible woody materials or rocks 	10 5 1

Monkey Pond Wetland Name/Code:

Evaluation Questions	Observations & Notes	Answers	Score
9. What is the abundance of floating & submerged vegetation?	Date of Observation: 7/21 Wetland was an est. 2 feet below high water line dur- ing time of site visit. Brase- nia schreberi present in small portion of open wa- ter	 a. Abundant: More than 70% of water area contains cover objects such as pond lilies, pondweed, and bladderwort b. Moderately abundant: From 30 to 70% of water area contains floating and submerged vegetation c. Scarce: Less than 30% of the water area contains floating and submerged vegetation 	10 5 1
 10. Are there artificial barriers to the passage of aquatic life? (e.g. dams, elevated culverts, bridge with a width less than the natural stream channel, road crossings, etc. along the stream reach associated with the wetland). 	Culverts were elevated dur- ing time of site visit, but ap- pear to be sumberged by typical water levels	 a. No artificial barrier(s) present. b. An artificial barrier is present and equipped with a fish ladder or other provisions for fish passage, <u>or</u> artificial barrier is only present during extreme low water c. Dam, elevated culverts or other artificial barrier(s) is present without provisions for fish passage d. Stream not present 	10 5 1
11. Are fish or aquatic species present that are rare, threatened, endangered or "Species of Greatest Conservation Need"?	None identified by Natural Heritage Bureau, and none observed in the field	 a. Documented occurrence of a rare or endangered fish or aquatic life species within or immediately adjacent to the subject wetland b. Documented occurrence of a rare or endangered fish or aquatic life species within .5 miles of wetland and suitable habitat exists for this species within the wetland c. No documented occurrence of a rare or endangered fish or aquatic life species within .5 miles of wetland c. No documented occurrence of a rare or endangered fish or aquatic life species within .5 miles of wetland, but suitable habitat exists and wetland is within range of one or more rare species d. No documented occurrence of a rare or endangered fish or aquatic life species within .5 miles of wetland, and suitable habitat is not known to exist 	10 5 1

AVERAGE SCORE FOR FISH & AQUATIC LIFE HABITAT

(Add scores for each question and divide by 11)

Wetland Name/Code: _____

_____ Evaluation Date.July 21, 2020 ____ Evaluator:_____

4 – SCENIC QUALITY

Waukewan St/beach parking lot

Primary viewing Site: ____

	Evaluation Questions	Observations & Notes	Answers	Score
1.	How many wetland vegetation classes are visible from the primary viewing location(s)? Refer to Appendix F for more information about wetland vegetation classes.		a. Three or more classesb. Two classesc. One class	10 5 1
2.	Is there public access at the viewing site?	Wetland visible from rail- road, road, and parking lot for popular swimming area	 a. Viewing site is on a property with public access, and trails to the site, or site is along a road. b. Wetland is on property with public access but <u>no</u> trails to the site. c. Wetland is on a property that does not have public access. 	10 5 1
3.	What is the visible extent across the wetland?		 a. Large expanse visible and low growing plants, or mixed vegetation classes you can see through b. View is somewhat restricted by trees and shrubs c. Forested or scrub-shrub wetland with little or no expanse visible. 	10 5 1
4.	What is the approximate extent of open water (including streams) visible from the primary viewing location/s?	Open water visible from road	a. More than 3 acresb. From 1 to 3 acresc. Less than 1 acre	10 5 1
5.	Does the wetland provide visual contrast with the surrounding landscape?	Lake Waukewan provides striking contrast	 a. High level of visual contrast with surrounding natural landscape. b. Some visual contrast with surrounding natural landscape c. Little visual contrast with surrounding landscape, or surrounding landscape is developed 	10 5 1
6.	What is the general appearance of the wetland and surrounding land use(s) visible from primary viewing location(s)?	No visual detractors	 a. Wetland is undisturbed and natural. No visual detractors, such as buildings, litter, abandoned cars, or powerlines b. Limited disturbance in and/or around wetland. Minor visual detractors c. Severe visual detractors present 	10 5 1

AVERAGE SCORE FOR SCENIC QUALITY

(Add scores for each question and divide by 6)

7.7

Monkey Pond Wetland Name/Code:_____ Evaluation Date:_____

Evaluator: CB, GG

5 – EDUCATIONAL POTENTIAL

Parking lot at beach on Waukewan Street

	Evaluation Questions	Observations & Notes	Answers	Score
1.	What is the Ecological Integrity of the wetland?		Average Score from 1- Ecological Integrity	5
2.	Does the wetland have high value wildlife habitat?		Average Score from 2 – Wetland-Dependent Wildlife Habitat	
3.	Does the wetland have high value fish and aquatic life habitat?		Average Score from 3 – Fish & Aquatic Life Habitat	3.2
4.	Is all or part of the wetland on public or private property that has public or private access (i.e. with written permission)?		 a. Wetland is on a property with public or private access and trails to the site. b. Wetland is on a property with public or private access but <u>no</u> trails to the site. c. Wetland is on a property that does not currently have public or private access. 	10 5 1
5.	How close is the educational site to off- road parking suitable for 5-10 vehicles or large enough for a school bus?	Adjacent parking lot	 a. Adequate parking is available less than a 5 minute walk from the educational site. b. Adequate parking is a 5-15 minute walk from educational site, or parking is limited to less than 5 cars. c. Adequate parking is more than 15 mins walk from the educational site, or no adequate parking is available. 	10 5 1
6.	How many wetland vegetation classes are accessible or potentially accessible for study at the educational site? Refer to Appendix F for more information about wetland vegetation classes.	PSS & PEM	a. Three or more wetland vegetation classesb. Two wetland vegetation classesc. One wetland vegetation class	10 5 1
7.	Is there access to open water (include streams) associated with the wetland at educational site?	Access not feasible	 a. Direct access to water available b. Water access is a short distance (5 mins or less) from the educational site c. No access or access not feasible d. No open water 	10 5 1 0
8.	What is the aesthetic and visual quality of the educational site?		Average Score from 4 – Scenic Quality	7.7
9.	Is the educational site accessible to the disabled?	Only if limited to parking lot	a. Yes b. No	10 0

(Add scores for each question and divide by 9)

Wetland Name/Code: Monkey Pond

July 21, 2020 ___ Evaluation Date:_____

Evaluator: <u>CB</u>, GG

6 – WETLAND-BASED RECREATION (CANOEING, KAYAKING, AND WILDLIFE OBSERVATION)

	Evaluation Questions	Observations & Notes	Answers	Score
1.	Are there opportunities for wildlife observation?	Limited visibility of open wa- ter is poor from road/parking lot	Average score for 2 – Wetland-Dependent Wildlife Habitat	4
2.	Is there access to suitable open water for canoes and kayaks?	No, though a nearby landowner expressed that open water was once more expansive and accessible to paddlers	 a. Open water is present, with easy access b. Open water is present, but site is not easily accessed for canoes/kayaks. c. Open water is present but no access is allowed or possible d. No open water suitable for canoe/kayak 	10 5 1 0
3.	Are there trail-based recreation opportunities?	No	 a. Maintained trails are present in and immediately adjacent to the wetland b. Trails are present but not maintained c. No trails are present 	10 5 1
4.	Are there off-trail recreation opportunities?	Access limited	 a. Wetland has open water greater than 0.5 acres in size AND an undisturbed 500 ft buffer for greater than 75% of the wetland edge. b. Wetland has open water greater than 0.5 acres in size OR an undisturbed 500 ft buffer for greater than 75% of the wetland edge. c. Wetland has neither open water nor an undisturbed buffer greater than 75% d. No access to potential recreation site or access not feasible 	10 5 1 0
5.	Is there off-road public parking at the potential recreation site for at least two cars?		 a. Adequate parking is available less than 5 minutes from the recreation site. b. Adequate parking is a 5-10 minute walk from the recreation site, or parking is limited. c. Adequate parking is more than 10 minutes walk from the recreation site, or no adequate parking is available. d. No access to potential recreational site or access is not feasible 	10 5 1 0
6.	What is the scenic quality of the potential recreational site?		Average score from 4 – Scenic Quality	6.1

AVERAGE SCORE FOR WATER-BASED RECREATION (Add scores for each question and divide by 6)

Monkey Pond

Wetland Name/Code:____

July 21, 2020 Evaluation Date: CB, GG Evaluator:

7 – FLOOD STORAGE

Instead of manually calculating the Wetland Flood Index on this data sheet, you can use the Flood Index Worksheet, an Excel spreadsheet provided on the <u>NH Method website</u> which is set up to do all the calculations for you. An example of the spreadsheet is provided in Table 3.

Note that this function is scored somewhat differently from the other NH Method function. A series of factors are developed that are then use to derive the Flood Storage Index. The numerical scores for the factors <u>do not correspond</u> to the 10, 5, 1, 0 scoring scale used in the other functions.

In the following situations, the Flood Value Index does not need to be calculated for the wetland being studied. Instead a certain flood index range can be assumed:

- 1. Wetlands with slopes greater than 10% (10' vertical :100' horizontal) as measured along the flow path, where it is obvious that little flood attenuation could occur, should be assigned a Low Flood Index Value range (0.0 to 0.9).
- 2. For large ponds or lakes or wetlands with ponded water surface area greater than 200 acres and streams that are Fourth Order or higher (i.e. 4th, 5th, 6th etc.) assign a High Flood Index Value range (7.6 to 10.0)

Evaluation Questions	Observations and Notes	Answers	Factor
1. What is the Wetland Acreage (W)? Be sure to EXCLUDE the acreage of any upland islands from the total wetland acreage		acres	
2. What is the Watershed Acreage (S)?		8,307 acres	
3. What is the Water Storage Depth in the wetland (D)?		 a. Use the actual water storage depth if known b. Assign a default value of 1.0 if the wetland is located in a 100 year floodplain c. Assign a default value of 1.0 ft if the actual water storage depth is not known 	$D = \frac{3}{1000} \text{ ft}$ D=1.0 ft D=1.0 ft
 What is the Wetland Storage Volume (V)? 		Multiply Water Storage Depth by Wetland acreage: D x W = V	v <u>5</u> .2 a5re feet
5. Wetland Storage Volume Factor (F)		Insert value from Table 1	F≐ ⁴⁵
6. Watershed Area Factor (A)		Insert value from Table 2	A = ^{.5}
7. Location of wetland within the watershed (L) (Choose the highest factor that applies)		 a. Wetland located within 1,000 ft of a 4th order or higher stream OR within 1000 ft of a pond/lake that outlets to a 4th order or higher stream b. Wetland located within 500 ft of a perennial stream (less than 4th order) c. Neither of the above situations apply to the study wetland 	1.0 0.8 0.6

SCORE FOR WETLAND FLOOD INDEX = F x A x L x 10

1.8

Use the score to locate the Value Range below and assign Flood Index Value

Wetland Flood Index Value	es Flood Value Type
0.0-0.9	Low Flood Value
1.0 - 2.5	Low to Moderate Flood Value
2.6 - 5.0	Moderate Flood Value
5.1 – 7.5	Moderate to High Flood Value
7.6 - 10.0	High Flood Value

Monkey Pond Wetland Name/Code:___ July 21, 2020 Evaluation Date:_____

Evaluator.^{CB, GG}

TABLE 1*				
Wetland Storage V	olume Factor (F)			
Wetland Storage Volume (V) (acre-feet)	Value of F			
≥ 200	1.000			
150	0.950			
100	0.900			
75	0.850			
50	0.800			
37.5	0.750			
25	0.700			
18.75	0.650			
12.5	0.600			
9.375	0.550			
6.25	0.500			
4.69	0.450			
3.125	0.400			
2.36	0.350			
1.6	0.300			
1.2	0.250			
0.8	0.200			
0.6	0.150			
0.4	0.100			
0.3	0.075			
0.2	0.050			
0.15	0.037			
0.1	0.025			
0.05	0.012			
0	0.000			

TABLE 2*				
Watershed Area Factor (A)				
(P) Wetl. Area/Wshed Area x 100	Value for A			
≥10%	1.00			
9%	0.95			
8%	0.90			
7%	0.85			
6%	0.80			
5%	0.75			
4%	0.70			
3%	0.65			
2%	0.60			
1%	0.55			
< 1%	0.50			

*(you will need to interpret your value to the closest value in Tables 1 and 2) SEE BELOW LEFT FOR EXAMPLES OF WETLAND FLOOD INDEX CALCULATION:

Example 1: (See Wetland I.D. 1 in Table 3 – sample spreadsheet) Wetland Area (W) = 0.25 acres Watershed Area (S) = 25 acres Water Storage Depth (D) = 0.5 ft (known depth) Water Storage Volume (V) = 0.5 ft x 0.25 acres = 0.125 acre-feet Wetland Storage Volume Factor (F) = 0.03 (from Table 1) Watershed Area Factor (A) = 0.55 (from Table 2, where 0.25 acres/25 acres x 100 = 1%) Location in Watershed (L) = 0.8 Wetland Flood Index = 0.03 x 0.55 x 0.80 = 0.0132 Flood Value Type = Low Flood Value

Example 2: (see Wetland I.D. W3 in Table 3 – sample spreadsheet) Wetland Area (W) = 33 acres Watershed Area (S) = 17,937 acres Water Storage Depth (D) = 1.0 ft (default value) Water Storage Volume (V) = 1.0 ft x 33 acres = 33 acre-feet Wetland Storage Volume Factor (F) = 0.73 (from Table 1) Watershed Area Factor (A) = 0.5 (from Table 2, where 33 acres/17,937 acres x 100 = 0.18%) Location in Watershed (L) = 1.0 Wetland Flood Index Value Type = 0.73 x 0.5 x 1.0 = 3.65 Flood Value = Moderate Flood Value

Monkey Pond

Evaluation Date:_____

Evaluator: <u>CB</u>, GG

Table 3: Example of Flood Index Worksheet for Multiple Wetlands

*Use the Excel spreadsheet on the <u>NH Method Website</u>

for automated calculation of the Flood Water Storage Index

"Red" headings indicate data input columns

Wetland Name/Code:_

Flood Index = (F x A x L) x 10 Where: Maximum Wetland Storage Volume = 200 acre-ft Maximum Wetland Flood Function Value = 10

"Black" headings indicate columns where the figures are automatically calculated

Wetland I.D.	Wetland Acreage	Watershed Acreage	Wetland Area as % of	Watershed Area Factor	Location in Watershed	Water Storage Depth	Wetland Storage Volume	Wetland Storage Volume	Flood Index
	(W)	(S)	Watershed (P)	(A)	(L)	feet (D)	acre feet (D)	Factor (F)	
	(00)	(3)	(F) from Table 2	Table 2	(L) (1.0/0.8/0.6)	(D) 1.0 = default	acre feet	(F) Table 1	
					(1.0/0.0/0.0/	1.0 - actaur			
1	0.25	25	1.00	0.55	0.8	0.5	0.125	0.03	0.132
2	0.75	15	5.00	0.75	1	1	0.75	0.19	1.425
3	2	50	4.00	0.7	0.8	2.5	5	0.46	2.576
4	10	100	10.00	1	1	3	30	0.72	7.200
5	10	1000	1.00	1	1	4	40	0.77	7.700
6	3	47	6.38	0.81	0.8	2	6	0.48	3.110
7	0.1	3	3.33	0.42	0.6	0.5	0.05	0.016	0.040
8	0.75	20	3.75	0.68	0.6	0.15	0.1125	0.027	0.110
9	1	50	2.00	0.6	1	2.5	2.5	0.35	2.100
10	50	400	12.50	1	0.8	3	150	0.95	7.600
W1	283	19548	1.45	0.57	1	1	283	1	5.700
W3	33	17937	0.18	0.5	1	1	33	0.73	3.650
W4	54	17291	0.31	0.5	1	1	54	0.73	3.650
W5	202	16619	1.22	0.56	1	1	202	1	5.600
W6	175	2664	6.57	0.82	1	1	175	0.95	7.790
W7	40	446	8.97	0.94	1	1	40	0.78	7.332
W8	24	380	6.32	0.51	1	1	24	0.69	3.519
W9	43	679	6.33	0.51	1	1	43	0.77	3.927
W10	116	2161	5.37	0.77	1	1	116	0.92	7.084
W11	63	880	7.16	0.86	1	1	63	0.83	7.138
W12	24	3302	0.73	0.86	1	1	24	0.69	5.934
ND1	93.7	5169	1.81	0.57	1	1	93.7	0.88	5.016
ND2	50	3741	1.34	0.57	1	1	50	0.8	4.560
ND3	37	258	14.34	1	1	1	37	0.75	7.500
ND4	101	2700	3.74	0.68	1	1	101	0.9	6.120
ND5	110.5	562	19.66	1	1	1	110.5	0.92	9.200
ND6	99	1753	5.65	0.77	1	1	99	0.9	6.930

Monkey Pond

Wetland Name/Code:_

July 21, 2020 Evaluation Date:

Evaluator: <u>CB, GG</u>

8 – GROUNDWATER

Note that this function does not require any field work

	Evaluation Questions	Observations & Notes	Answers	Score
1.	Does the wetland overlie a stratified drift aquifer?	Wetland is within 1/4 mile from a stratified drift aquifer (see at- tached map)	 a. Wetland overlies a stratified drift aquifer b. Wetland is within ¼ mile of a stratified drift aquifer c. Wetland is more than ¼ mile from a stratified drift aquifer 	10 [,] 5 1
2.	Is the wetland in a potential public water supply area?	Yes Lake Waukewan is the public drinking water supply for Meredith	 a. Wetland is in an area identified by Favorable Gravel Well Analysis b. Wetland is within ¼ mile of an area identified by Favorable Gravel Well Analysis c. Wetland is more than ¼ mile from an area identified by Favorable Gravel Well Analysis 	10 5 1
3.	Is the wetland within a public wellhead protection area?	No	 a. More than 75% of the wellhead protection area includes the wetland b. 25%-75% of the wellhead protection area includes the wetland c. Less than 25% of the wellhead protection area includes the wetland 	10 5 1
4.	What is the percent coverage of highly permeable soils within 100 ft of the wetland? Refer to Table 3 to answer this question	<25%	 a. More than 50% of the soil types within 100 ft of the wetland are on the list in Table 3. b. 25-50% of the soil types within 100 ft of the wetland listed in Table 3 c. Less than 25% of soil types within 100 ft of the wetland are listed in Table 3 	10 5 1
5.	What is the percent coverage of the highly permeable soil types listed in Table 4 within the wetland? Refer to Table 4 to answer this question	<25%	 a. More than 50% of the soil types within the wetland are on the list in Table 4 b. 25-50% of the soil types within the wetland listed in Table 4 c. Less than 25% of the soil types within the wetland are listed in Table 4 	10 5 1

AVERAGE SCORE FOR GROUND WATER

(Add scores for each question and divide by 5)

Wetland Name/Code: Monkey Pond

Evaluation Date: July 21, 2020

CB, GG

Table 3: SAND & GRAVEL SOIL TYPES

Note: This list of soils was prepared for the purpose of providing an additional data layer for consideration under the groundwater function – i.e. to include areas that are not mapped as aquifer recharge areas yet contain surface soils with coarse particle sizes which enhance infiltration.

Number & Slope Classes ¹	Map Unit name & Particle Size Groups ²	Drainage Class ³	Record % of 100- ft. wetland buffer
12 B,C,D	Hinckley gravelly LS	ED	
21 B,C,D	Colton, gravelly LS	ED	
22 B,C,D	Colton LS	ED	
24 B,C	Agawam FSL & LS	WD	
25 B,C,D	Ninigret-Windsor complex LS	MWD/WD	
26 B,C,D	Windsor LS	ED	
35 B,C,D	Champlain LS	SED	
36 B,C,D	Adams LFS	SED	
22 A,B,E	Colton S&G	ED	
212 B,C	Hinckley, very gravelly LS	ED	
222 B,C,D	Colton, very stony LS	ED	
236 B,C,D	Adams, very stony FLS	SED	
300	Udipsamments	SED	
313	Deerfield, LS	MWD	
350	Udipsamments	SED	
400	Udorthents, S	ED	
526 B,C	Caesar LS	ED	

1. SLOPE CLASSES

A, B = 0 – 8% (includes 'A' on older maps) C = 8 – 15% D = 15 – 25% E = > 25%

2. PARTICLE SIZE GROUPS

```
F = fine L = loam S = sand
```

3. DRAINAGE CLASSES

WD = well drained SED = somewhat excessively drained ED = excessively drained MWD = moderately well drained

SL = sandy loam

G = gravel

LS = loamy sand

Monkey Pond

Wetland Name/Code:______ Evaluation Date:_July 21, 2020 Evaluator:_CB, GG

Table 4: HIGHLY PERMEABLE WETLAND SOIL TYPES THAT POTENTIALLY CONTRIBUTE TO RECHARGE DURING DRY SEASONS.

Мар	Soil Name		Draina	ge Class	
Symbol		Somewhat	Poorly	Very Poorly	Record % of
•		Poorly Drained	Drained	Drained	wetland area
15	Searsport			Х	
34	Wareham		Х		
115	Scarboro			Х	
125	Scarboro, very			Х	
	stony				
214	Naumberg		Х		
314	Pipestone		Х		
315	Mashpee		Х		
325	Scarboro variant			Х	
326	Scarboro variant,			Х	
	very stony				
393	Timakwa			Х	
394	Chocorua variant			Х	
395	Chocorua			Х	
433	Grange		Х		
546	Walpole		Х		
547	Walpole, stony		Х		
614	Kinsman		Х		
615	Augres		Х		
900	Endoaquents,		Х	Х	
	sandy				
913	Sudbury variant	Х			
914	Duane variant	Х			
915	Deerfield variant	Х			
916	Croghan variant	Х			
918	Madawaska	X			
	variant				
992	Pondicherry			Х	
				Total percent	%

Monkey Pond Wetland Name/Code:_____

Evaluation Date: July 21, 2020

CB, GG Evaluator:_____

9 – SEDIMENT TRAPPING

	Evaluation Questions	Observations &Notes	Answers	Score
1. What walk	at is the wetland's Flood Storage ue?		Average score from 7 – Flood Water Storage.	1.8
	es the wetland lack outlet or have a stricted outlet?	Outlets (culverts) have historically been plugged by beavers, according to a landowner, but were un- obstructed at time of visit	 a. Wetland has no outlet or has a constricted outlet or is ponded above the outlet b. Wetland has an outlet but flow path through wetland is primarily sheet flow c. Wetland outlet not constricted or flow primarily within stream channel. 	10 5 1
th	hat is the character of water flow rough the wetland? Channel Length t line distance of stream Channel Length t line distance of stream	440/343ft=1.3 sinuosity ratio	 a. At least one of the following situations apply: No stream channel OR Inlet present but no outlet OR Outlet is im pounded and standing water present in downstream end of wetland OR Inlet and outlet present and channel sinuosity is ≥ 1.5 b. Inlet and outlet present, and sinuosity of channel is >1.0 and <1.5 c. Channel is straight (sinuosity=1.0) and no impoundments within wetland or at wetland outlet 	10 5 1
to	hat is the ratio of the wetland's size the size of its watershed? Acres of Wetland x 100 Area of watershed above wetland outlet		 a. Wetland is more than 10% of its watershed b. Wetland is between 1-10% of its watershed. c. Wetland is less than 1% of its watershed. 	10 5 1
	hat is the gradient within the etland?		 a. Wetland has gradient < 0.5% or no outlet b. Wetland gradient is 0.5% to 3% c. Wetland has gradient greater than 3%. 	10 5 1
all tra scr em _{Refe}	hat is the areal extent (% coverage) vegetation types that will most likely ap sediments? (e.g. forested swamps, rub shrub swamps, and persistent nergent marshes) or to Appendix F for more information about and vegetation classes.	PEM classes cover about 75% of the wetland	 a. Persistent emergent plants (stems above surface of water /wetland throughout the year), trees and/or shrubs cover at least 90% of the surface area of the wetland. b. Persistent emergent, trees and/or shrubs, and/or non-persistent emergents (stems fall below the surface of water/wetland during fall and winter) cover 50-90% of the wetland's surface area. c. Persistent emergent, trees and/or shrubs, and/or shrubs, and/or shrubs (stems fall below the surface area). 	10
			and/or non-persistent emergents (stems fall below the surface of water/wetland during fall and winter) cover <50% of the wetland's surface area.	
	hat is the average water depth in the etland during growing season?		 a. Average water depth is < 1 ft or there is no open water b. Average water depth > 1 ft and < 6.6 ft. c. Average water depth is greater than 6.6 ft 	10 5 1

AVERAGE SCORE FOR SEDIMENT TRAPPING: (Add scores for each question and divide by 7)

Monkey Pond Wetland Name/Code:_____

Evaluation Date:_____

Evaluator: <u>CB, GG</u>

10 – NUTRIENT REMOVAL/RETENTION/TRANSFORMATION

	Evaluation Questions	Observations &Notes	Answers	Score
1.	What is the wetland's Flood Storage value?		Average score from 7 – Flood Storage.	1.8
2.	What is the wetland's ability to trap sediments?		Average score from 9 – Sediment Trapping.	4.1
3.	What is the extent (percent cover) of persistent emergent vegetation, trees and/or shrubs within the wetland?		Record answer from 9 – Sediment Trapping , Question 6	5
4.	What hydroperiod occurs over more than 50% of the wetland?		 a. Semi-permanently flooded, seasonally flooded/saturated, or saturated b. Seasonally flooded, seasonally flooded/well-drained or temporarily flooded c. Permanently flooded or intermittently exposed 	10 5 1
5.	What hydric soils cover the greatest percentage of the wetland?	The soils are not mapped as hydric by the NRCS, but were clearly hydric based upon field examination.	 a. Wetland is dominated by fine textured soils (refer to Table A, Appendix D) b. Wetland is dominated by organic and/or peat soils (refer to Table B, Appendix 3) c. Wetland is dominated by sands and gravels (refer to Table C, Appendix D) 	10 5 1

AVERAGE SCORE FOR NUTRIENT TRANSFORMATION

(Add scores for each question and divide by 5)

5.2

Wetland Name/Code:_____

July 21, 2020 Evaluation Date:

Evaluator:_____ CB, GG

11 – SHORELINE ANCHORING

If there is no stream, river, lake or pond within or adjacent to the wetland, leave this Function out of the evaluation.

	Evaluation Questions	Observations & Notes	Answers	Score
1.	What is the gradation of wetland vegetation types along the shoreline?	PEM, PAB, PSS	 a. Three or more wetland vegetation types present (PAB, PEM, PSS or PFO) b. Two wetland vegetation types present c. One wetland vegetation type present 	10 5 1
2.	What is the vegetation density in the wetland bordering watercourse, lake or pond?		 a. High: More than 90% woody or persistent vegetation cover b. Moderate: From 70-90% woody or persistent vegetation cover c. Low: Less than 70% woody or persistent vegetation cover 	10 5 1
3.	How wide is the wetland bordering the watercourse, lake or pond?		a. More than 20 feetb. From 10-20 feetc. Less than 10 feet	10 5 1
4.	How "rough" is the substrate of the wetland at the shoreline of the waterbody?		 a. Wetland substrate characterized by many boulders, stones or cobbles and woody material b. Wetland substrate has few boulders, stones or cobbles, or substrate is mostly gravel or coarse sands and little woody material c. Wetland substrate is uniformly smooth and is comprises of clays, silts or very fine sands or organic materials and no woody material 	10 5 1

AVERAGE SCORE FOR SHORELINE ANCHORING	7 5
(Add scores for each question and divide by 4)	

Wetland Name/Code: Monkey Pond

July 21, 2020 Evaluation Date:_____

CE Evaluator:

12 – NOTEWORTHINESS

Describe noteworthy features in the wetland narrative

Note that the scores for this function are totaled and NOT averaged

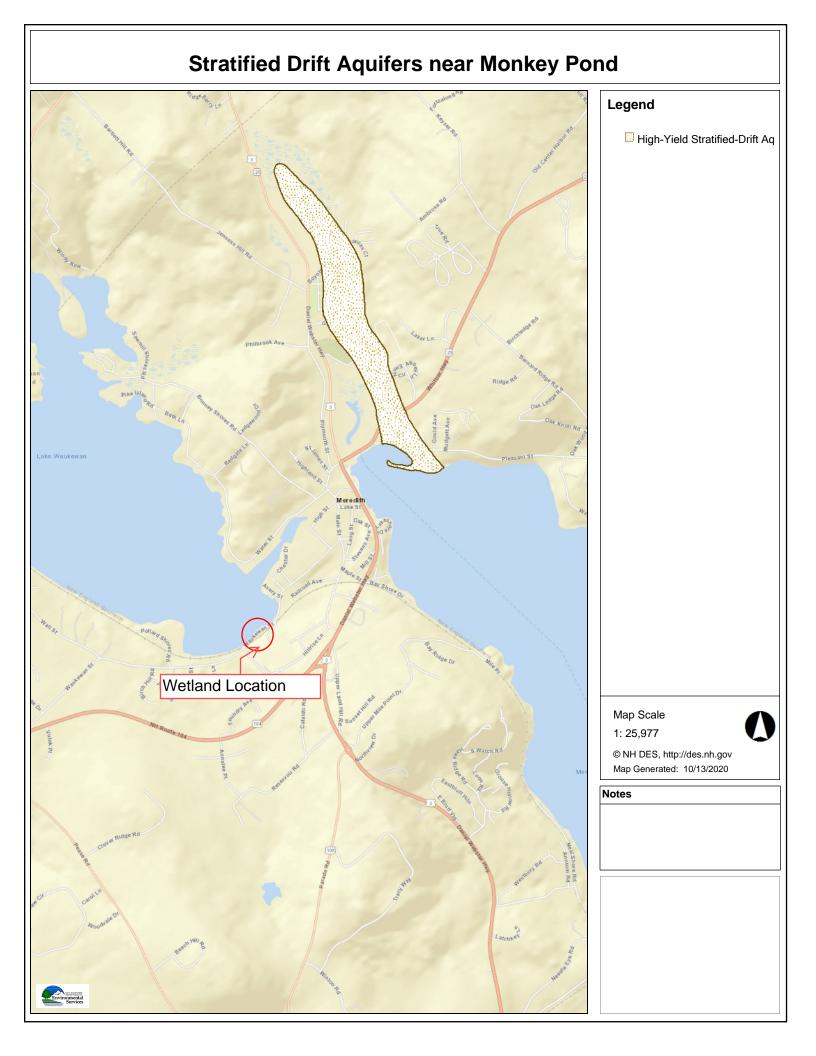
	Evaluation Questions	Observations & Notes	Answers	Score
1.	Is the wetland located in or within 500 ft of an area of Highest Ranked Habitat (state or regional level), as identified on the NH Wildlife Action Plan Highest Ranked Habitat Condition map?	Part of wetland located within highest ranked habitat	a. Yes	10
2.	Does the wetland have local significance because has consistently high scores for all functions and/or is among the top ten largest wetlands in town?		a. No	0
3.	Does the wetland have local, regional or statewide significance because it is it located in a priority area, is documented in a local or regional conservation plan, or it has been recognized as having regional importance in the state?		a. No	0
4.	Does the wetland have known biological, geological, or other elements that are rare or unique as documented by the NH Natural Heritage Bureau or as determined by a professional?	No (see attached results from NHB Data Check)	a. No	0
5.	Is the wetland known to contain a documented historical or archaeological site?	Reference the documentation here:		0
		No (see attached EMMIT review	a. No	0
		map, search conducted 10/8/20)		
6.	Is the wetland hydrologically connected to a state or federally designated river within ¼ mile of the wetland's outlet?		a. No	0
7.	Is the wetland one of just a few left in an urban setting?		a. No	0

TOTAL SCORE FOR NOTEWORTHINESS

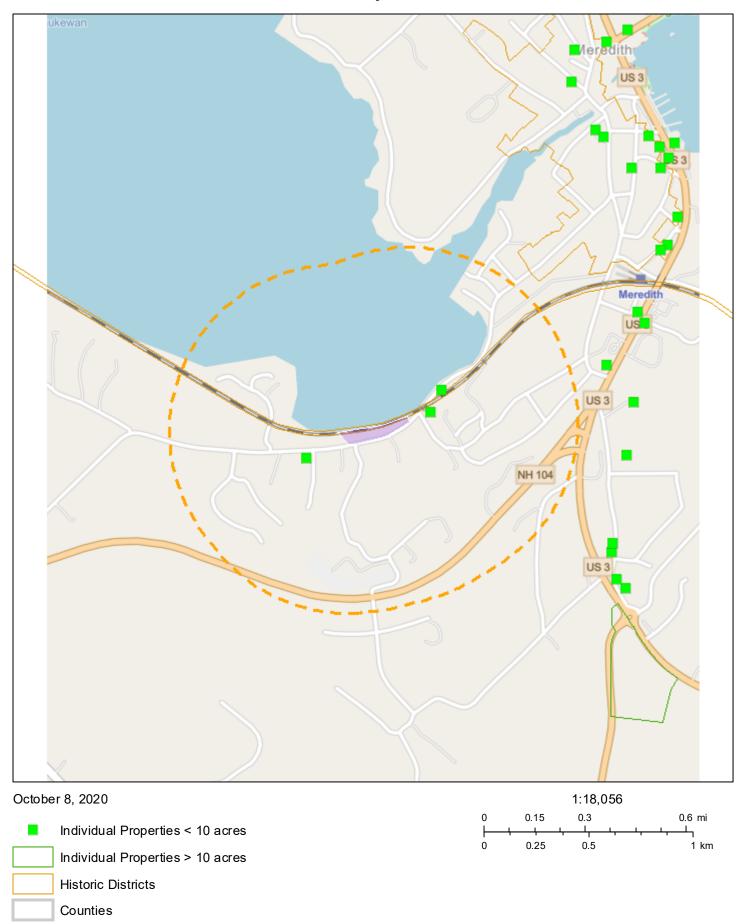
___10____

Add up the scores for all questions which received a YES answer.

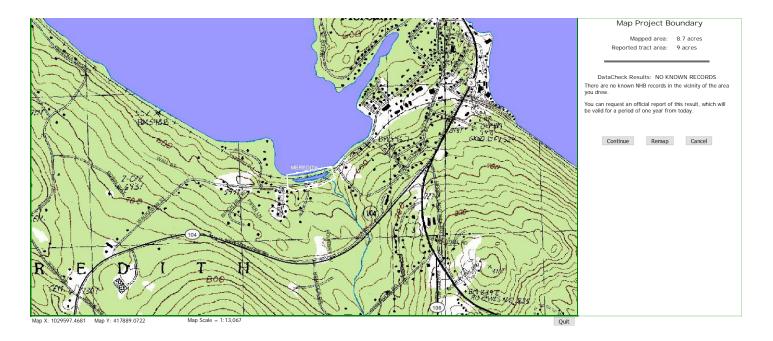
The total score is the score for this function (**note that this score is not averaged**). For example, if you answered YES to four questions, the score would be 40. If you answered YES to only one question, the score is 10 CB, GG



Monkey Pond



Towns



Monkey Pond, July 2020



Shallow pond #1 within wetland, east end.



Shallow pond #2 within wetland, west end, from road.



Shallow pond 2 within wetland, west end, from railroad.



Cattail/pickerelweed marsh in center of wetland.



Buttonbush in marsh in center of wetland.



Culvert (6' squash pipe) under Waukewan Road, carrying stream from south side of road into wetland.



Stream channel within wetland below 6' pipe.



Stream channel within wetland further to west.





Culvert from riparian corridor into wetland, west end.



Drop inlet for stormwater from road into wetland. Stormwater culvert outlet at wetland





Culvert outlet from wetland to south of road.



Culvert outlet from wetland to south of road.



Culvert, 4' concrete, under railroad.



Culvert, 4', metal, under railroad.



Stream channel with sediment deposition, south side Waukewan Rd, near road.



Stream channel with sediment deposition, South of Waukewan Rd, further upstream.



Stream channel with erosion and deposition, South of Waukewan Road, further upstream.



Stream channel furthest upstream.